

PROJECT SCOPE DECLARATION

THE FOLLOWING ALTERATIONS AND EXTERIOR ADDITIONS ARE PROPOSED AND DEFINED IN THE CONTENT OF THIS SUBMITTAL:

1) REPLACEMENT OF THE EXISTING SINGLE PITCHED GARAGE ROOF (NORTH END OF THE STRUCTURE) WITH A COVERED DECK. THE EXISTING GARAGE IS UNCONDITIONED AND NO MODIFICATION TO THE THERMAL ENVELOPE IS PROPOSED, BEYOND REQUIRED CONSTRUCTION ACCESS. EXISTING CAVITIES OPENED SHALL BE INSULATED PER 2018 WSEC R503.

2) THE INSTALLATION OF A NEW EXTENSION TO THE EXISTING ROOF LINE, TO PROVIDE COVER TO THE NEW NORTH DECK.

3) THE GARAGE WALL AND FOOTING MUST BE IMPROVED TO ADDRESS NEW LATERAL FORCES INTRODUCED BY THE NORTH DECK & ROOF EXTENSION.

4) THE EXISTING CONCRETE TILE ROOFING IS TO BE REPLACED THROUGHOUT WITH A RECYCLABLE & LONG LIFESPAN STANDING SEAM METAL ROOFING SYSTEM.

5) TWO NEW DOORS AT THE NEW NORTH DECK, AND THE REPLACEMENT OF THE DOOR AT THE PRIMARY ENTRANCE ARE REQUIRED. THE AREA WEIGHTED AVERAGE U-VALUE FOR ALL NEW FENESTRATION SHALL NOT EXCEED U=0.30. SEE A700

6) THE EXISTING EAST DECK AND STAIRS WILL BE REPLACED WITH NEW DECKING, FRAMING, BEAMS, AND COLUMNS. THE COLUMNS WILL CONNECT TO HELICAL PILES INSTALLED UNDER PERMIT 2012-223 USING THE STRUCTURAL ENGINEER'S DETAILING. ALL FINISHED WORK WILL BE INSTALLED WITHIN THE PLAN VIEW FOOTPRINT OF THE EXISTING DECK.

7) THE EXISTING PORTICO AT THE PRIMARY ENTRANCE IS TO BE REMOVED. IT WILL BE REPLACED BY A CONTINUATION OF THE PRINCIPAL ROOF SURFACE AND EXISTING EAVE LINE.

8) CONNECT NEW AND REPLACED GUTTERS TO EXISTING DOWNSPOUT LOCATIONS. NO NET INCREASE IN RAINWATER CATCHMENT AREA OR OTHER DEMAND ON EXISTING STORMWATER SYSTEM.

LAND USE CODE SUMMARY

- 1) NO INCREASE IN EXISTING LOT COVERAGE
- 2) EXISTING RETAINING WALL, WALKWAY AND STAIRS INSTALLED IN CONFORMANCE TO MICC 17.14.010
- 3) NO HEIGHT INCREASE PROPOSED

ENERGY CODE SUMMARY

- 1) ALL WORK CLASSIFIED AS ADDITION IS EXTERIOR OR UNCONDITIONED.
- 2) NO ADDITION TO THE THERMAL ENVELOPE PROPOSED, THEREFORE GOVERNING SECTION IS 2018 WSEC R503
- 3) NO ENERGY CREDITS REQUIRED FOR ALTERATION
- 4) ALL CAVITIES ACCESSED TO BE FULLY INSULATED
- 5) ALL FENESTRATION U=0.30 MAX (SEE A700)
- 6) ALTERATION OF MECHANICAL SYSTEMS LIMITED TO RE-ROUTING DIRECT VENT EXHAUST OF THE EXISTING GAS FIREPLACE APPLIANCE. NO CHANGE TO EXISTING HEATING, COOLING, OR VENTILATION SYSTEMS.
- 7) ALL PROPOSED LIGHTING TO BE HIGH EFFICACY

FIRE SUMMARY

- 1) SHALL COMPLY WITH ANY CONDITIONS ESTABLISHED BY THE FIRE MARSHALL'S REVIEW

FOUNDATION SYSTEMS

- 1) SEE STRUCTURAL DESIGN FOR PILE CONNECTIONS,
- 2) SEE GEOTECHNICAL REPORT FOR SUB-GRADE DESIGN CRITERIA.
- 3) SEE PERMIT 2012-223 FOR RECORD OF SYSTEM INSTALLATION AND ON SITE LOAD TESTING.

APPLICABLE CODES

ALL WORK SHALL FULLY COMPLY WITH THE FOLLOWING CODES, AS AMENDED BY THE AUTHORITY HAVING JURISDICTION:

- 2018 INTERNATIONAL RESIDENTIAL CODE
- 2018 INTERNATIONAL BUILDING CODE
- 2018 INTERNATIONAL MECHANICAL CODE
- 2018 UNIFORM PLUMBING CODE
- 2018 INTERNATIONAL FUEL GAS CODE
- 2018 WASHINGTON STATE ENERGY CODE
- 2018 INTERNATIONAL FIRE CODE

PROJECT INFORMATION

ADDRESS: 3206 74TH PLACE SOUTHEAST
MERCER ISLAND, WA 98040

PARCEL NUMBER: 130030-2288

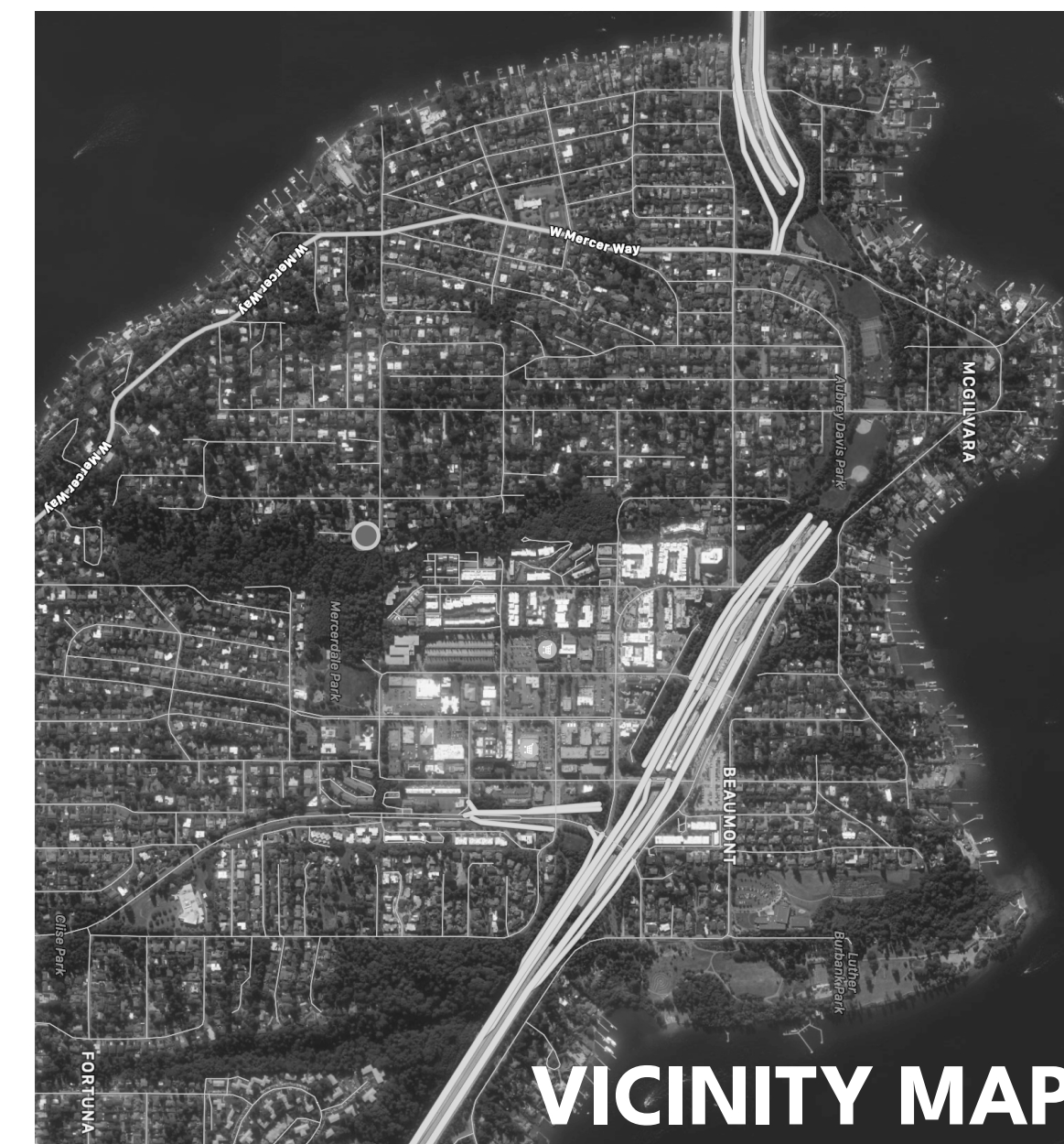
LEGAL DESCRIPTION: CALKINS C C 1ST TO EAST SEATTLE N 170 FT OF VAC CEDAR PLAZA LESS BEG AT NE COR TH S 100 FT TH NWLY TO NW COR TH E TO BEG

ZONING: R - 8.4

LOT AREA: 13,200 SQUARE FEET
NO INCREASE TO LOT COVERAGE OR IMPERVIOUS SURFACE AREA

LOT SLOPE: 17.5% (SEE SHEET A011)

CRITICAL AREAS: SLOPE HAZARD ON PARCEL, ALL PROPOSED WORK IS OUTSIDE OF SLOPE HAZARD AREA AND BUFFER.
SEE GEOTECHNICAL REPORT FOR CLARIFICATION OF MINIMAL RISK, CRITERIA & RECOMMENDATIONS.



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S5	ROOF FRAMING PLAN
S6	SECTIONS & DETAILS
S7	SECTIONS & DETAILS

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CATHERINEDEHAVEN@GMAIL.COM

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OWEN GOULD OWEN@OGENGINEERING.COM

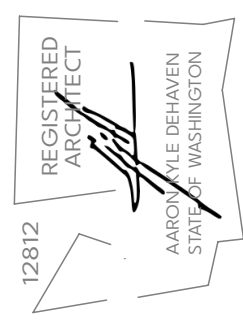
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PHIL HABERMAN COBALTGEO@GMAIL.COM

CONTRACTOR

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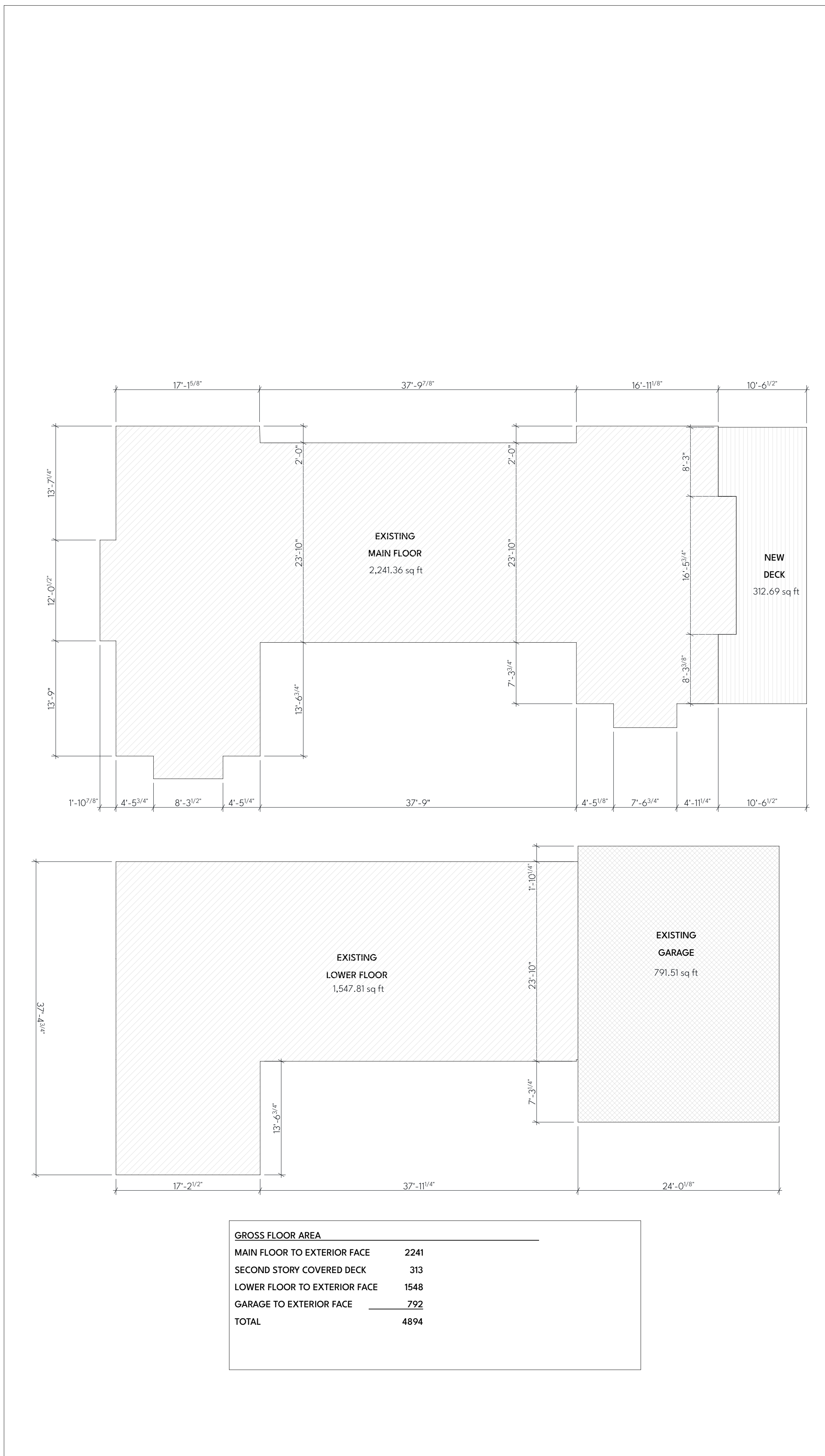


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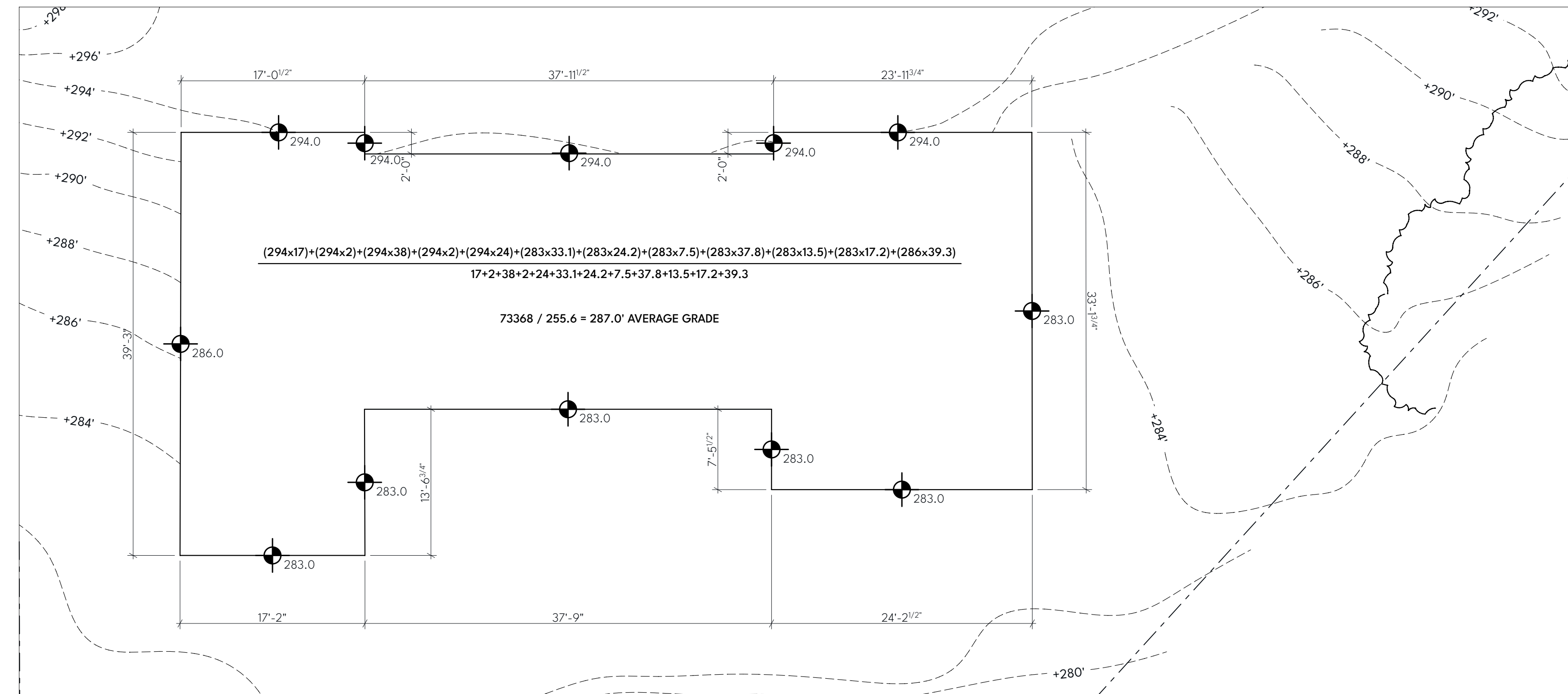
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FIRST HILL TREEHOUSE
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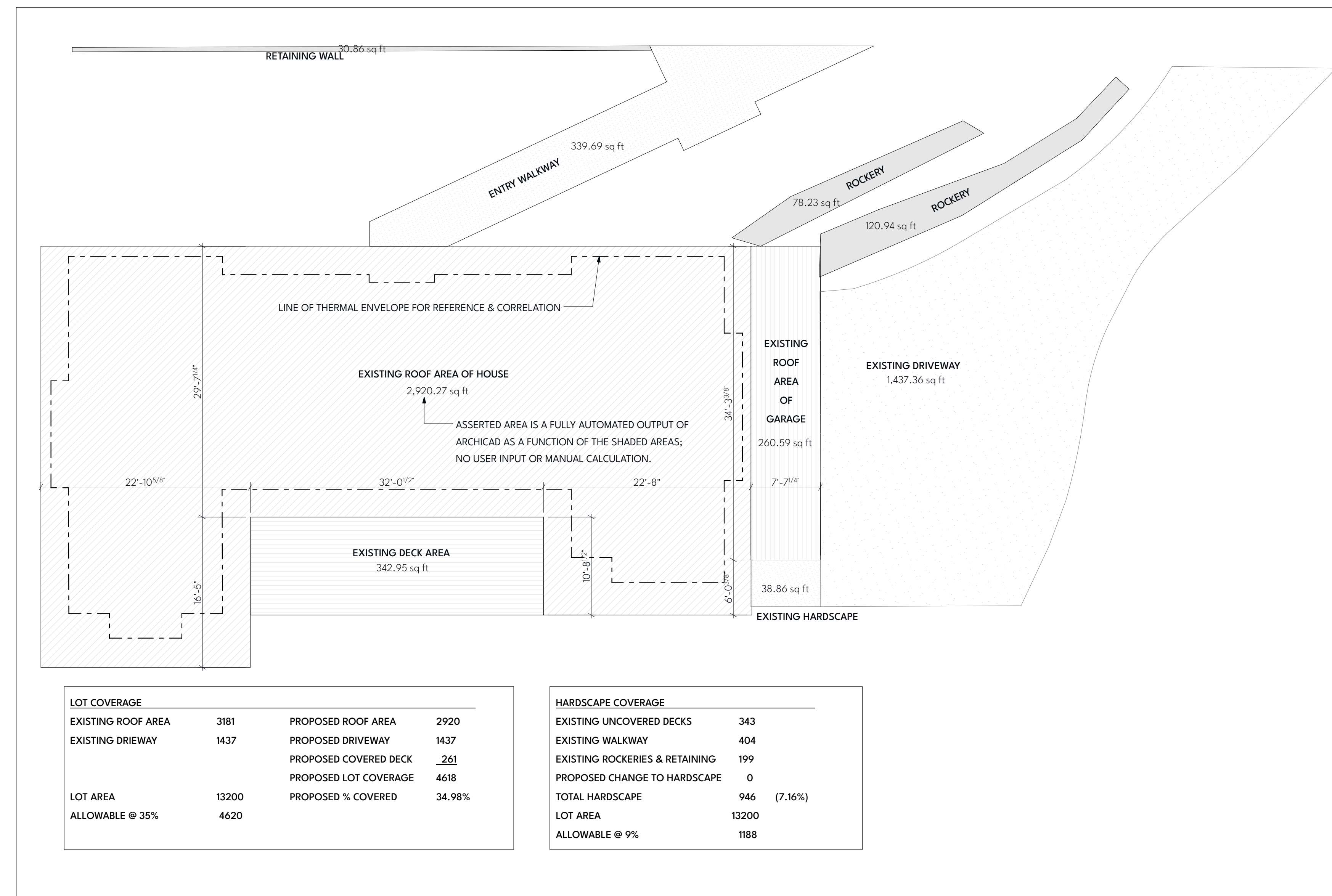
PROJECT INFORMATION
A001



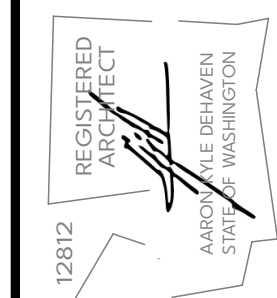
3 GFA AREA TAKEOFF
A002 SCALE: 1/8" = 1'-0"



1 AVERAGE GRADE
A002 SCALE: 1/8" = 1'-0"



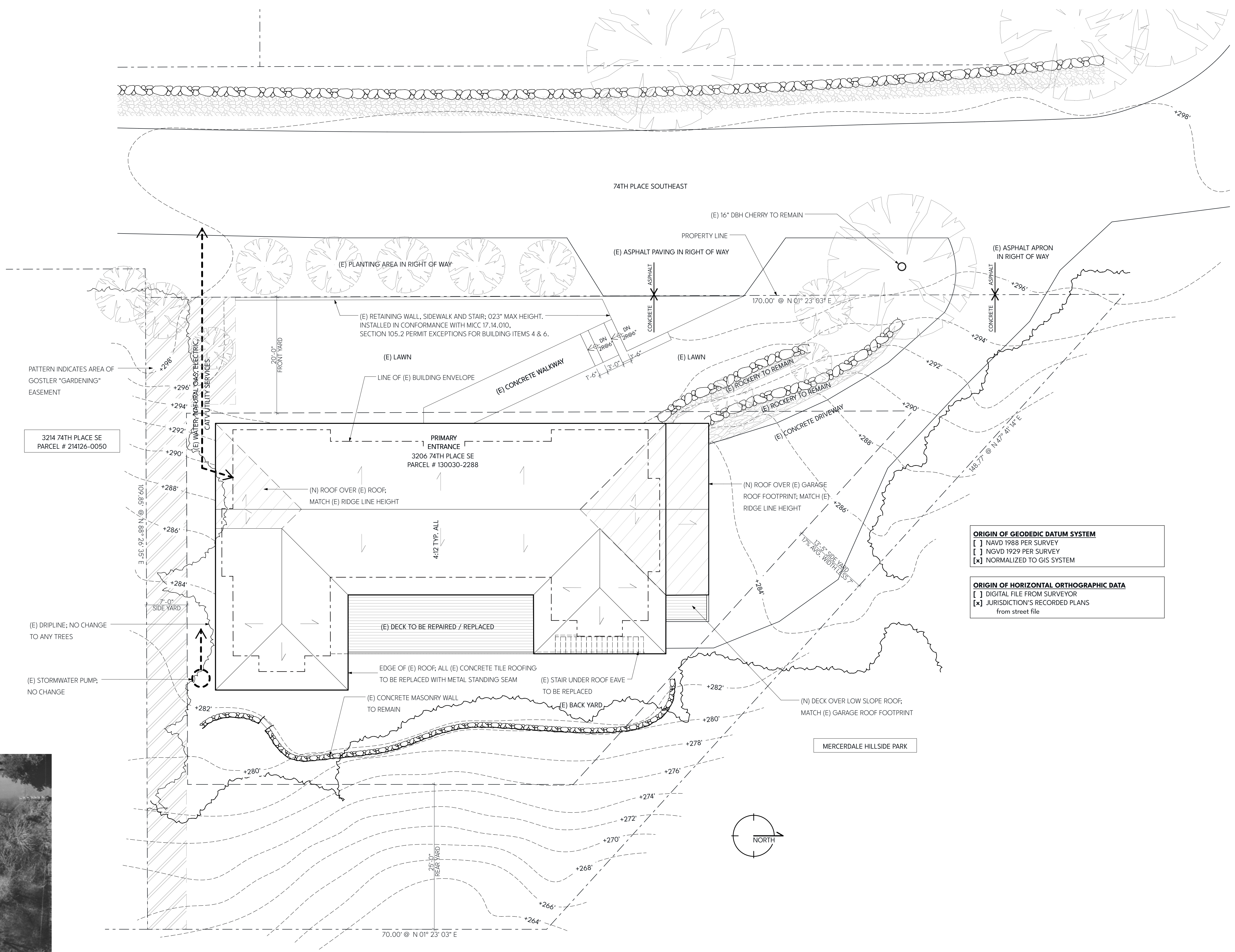
2 LOT COVERAGE
A002 SCALE: 1/8" = 1'-0"



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5	REVISION	
6	REVISION	

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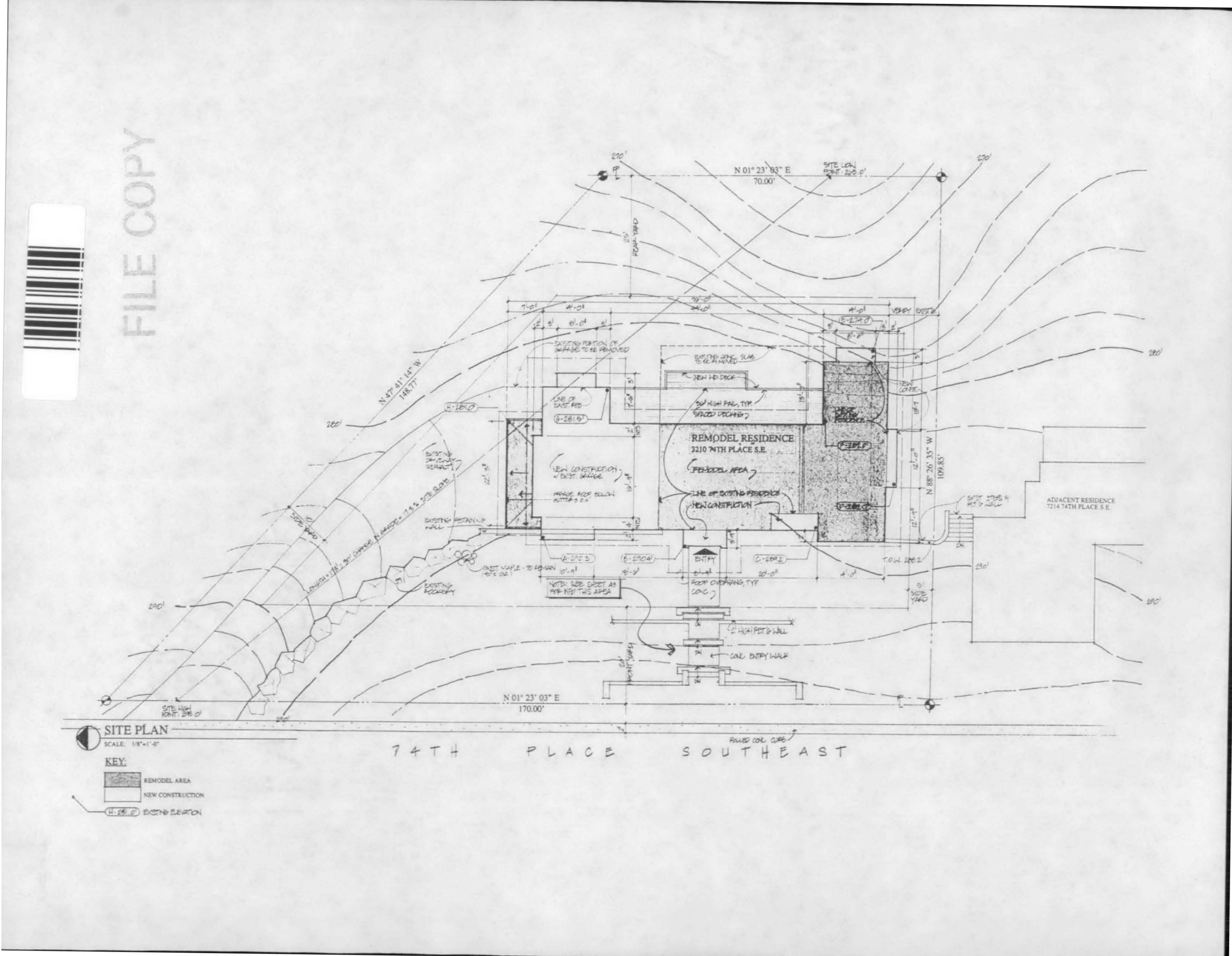
ORIGIN OF GEODESIC DATUM SYSTEM
 NAVD 1988 PER SURVEY
 NGVD 1929 PER SURVEY
 NORMALIZED TO GIS SYSTEM

ORIGIN OF HORIZONTAL ORTHOGRAPHIC DATA
 DIGITAL FILE FROM SURVEYOR
 JURISDICTION'S RECORDED PLANS
 from street file



2 AERIAL RASTER IMAGE
A010 SCALE: 1/32" = 1'-0"

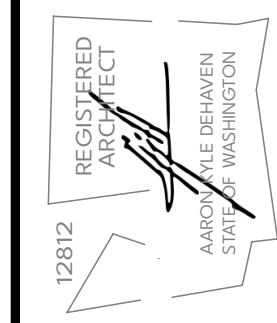
1 SITE PLAN
A010 SCALE: 1/8" = 1'-0"



FILE COPY

1 CITY RECORD SITE PLAN
A011 NOT TO SCALE

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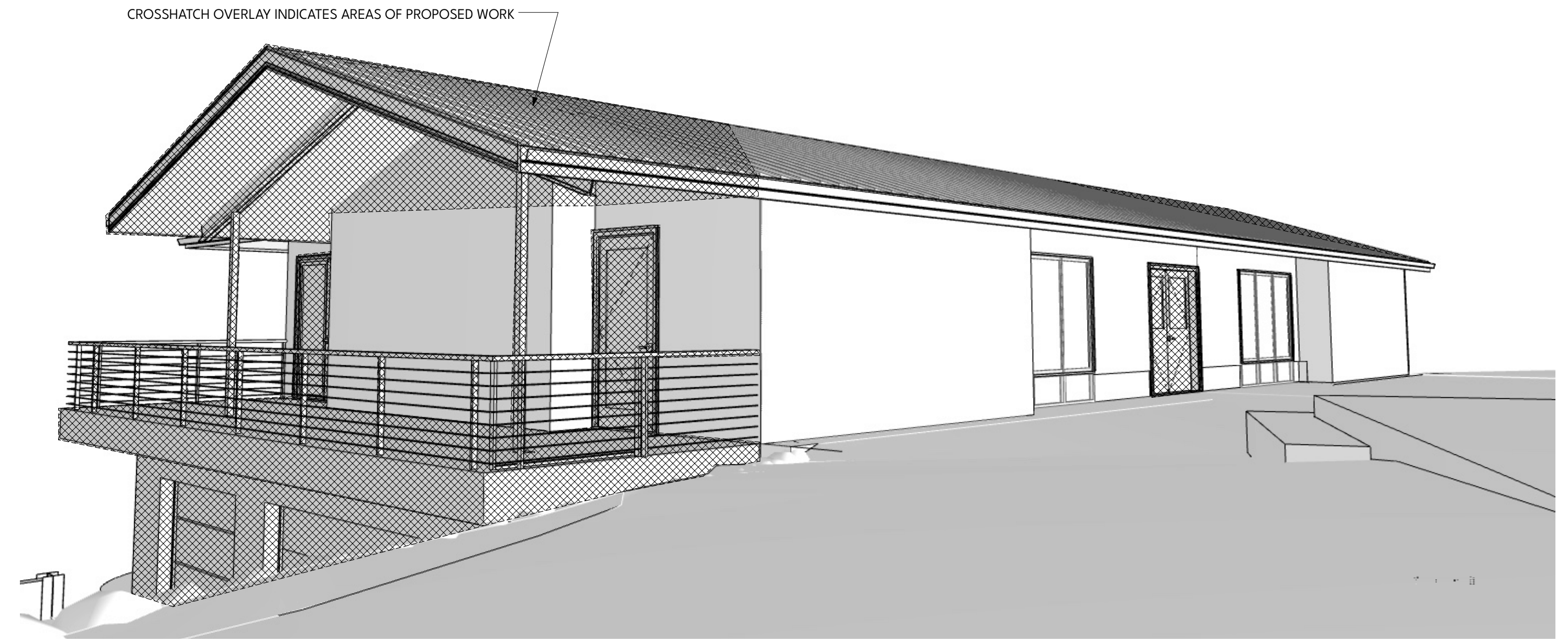
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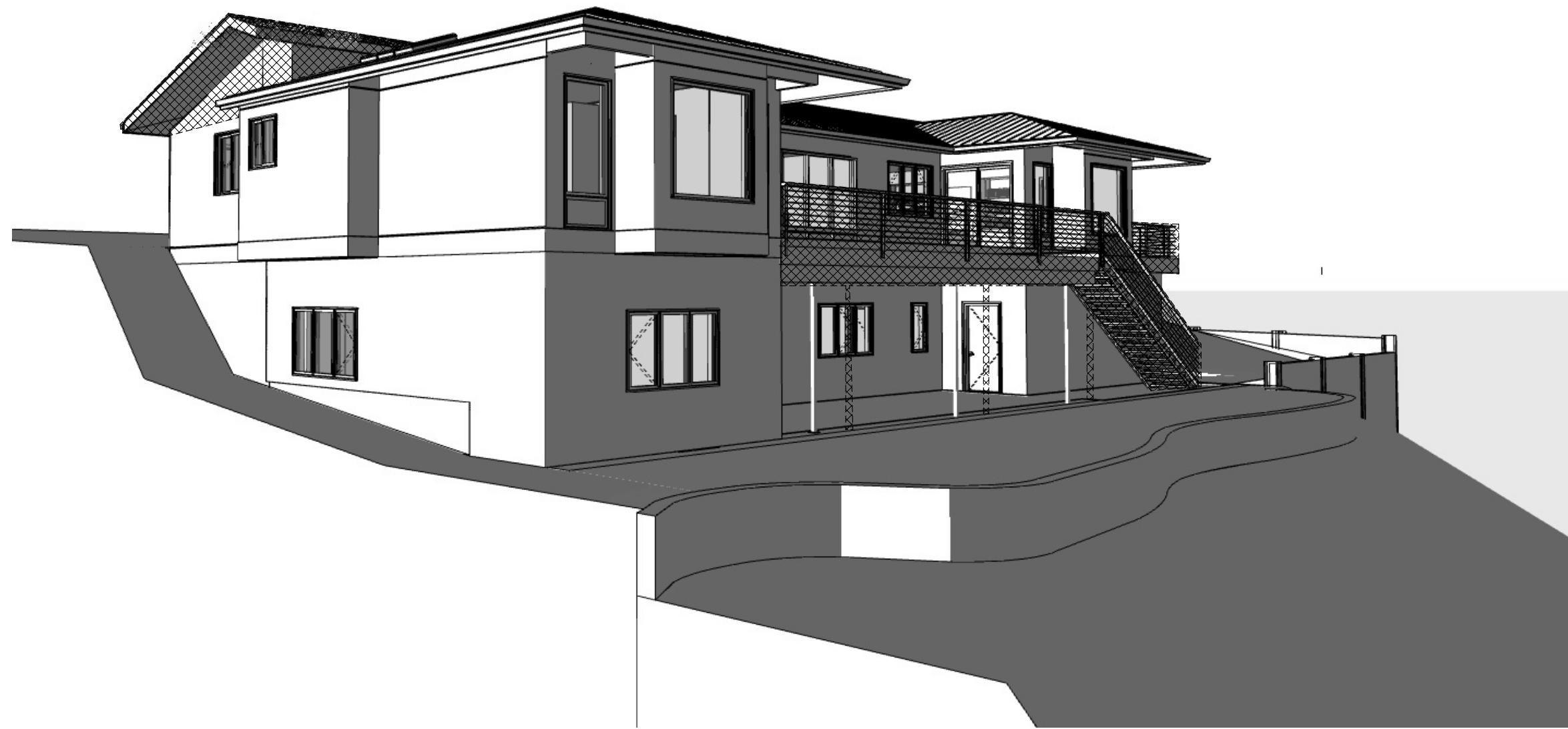
CITY RECORD SITE PLAN
A011



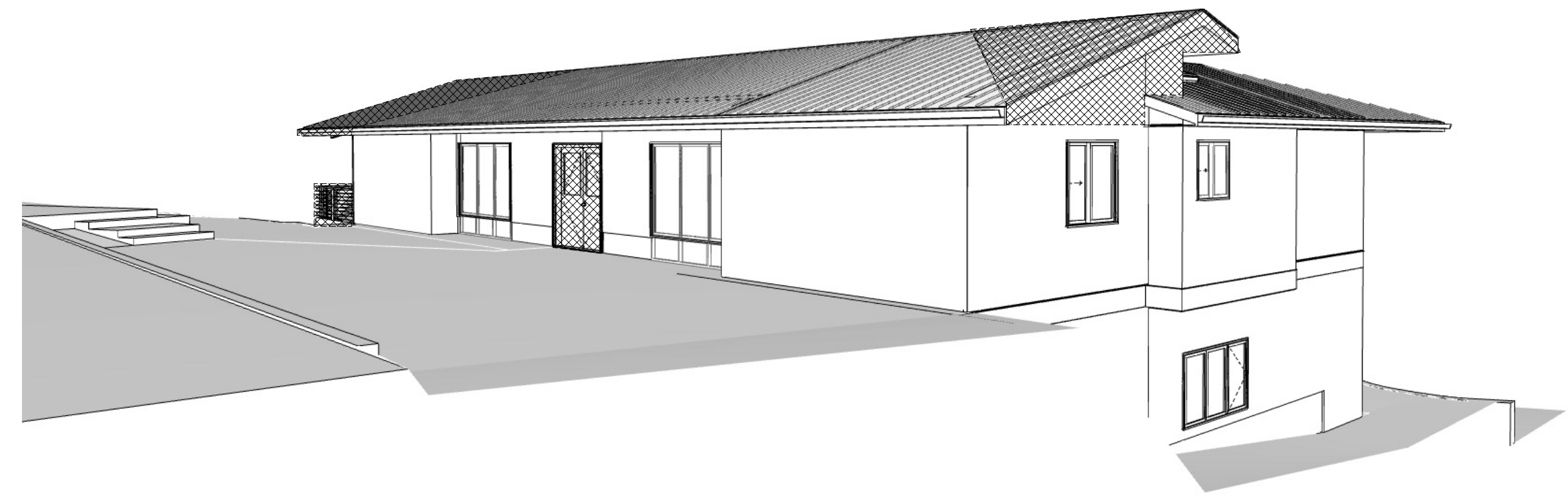
1 FROM NW
A020



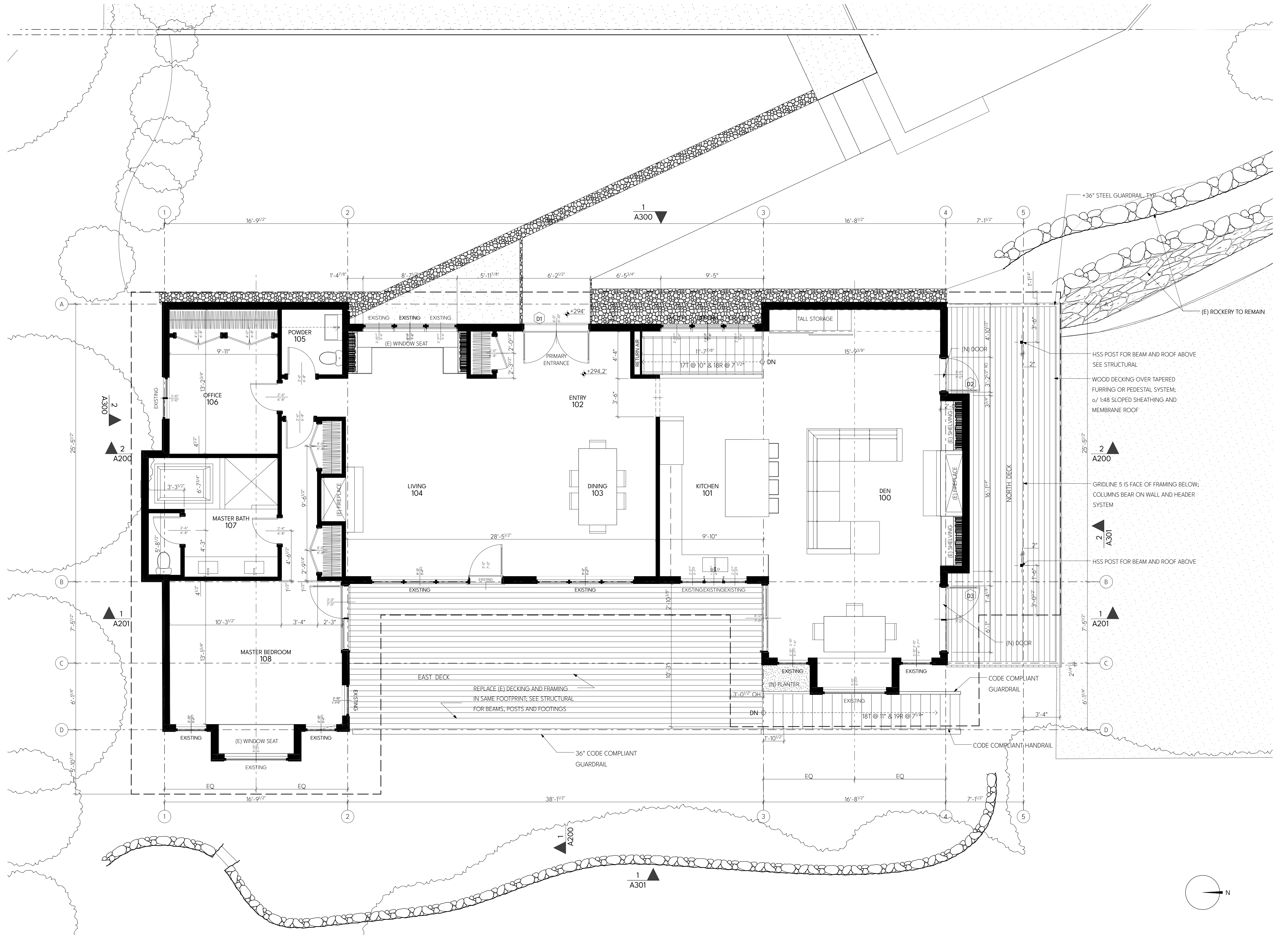
4 FROM NE
A020



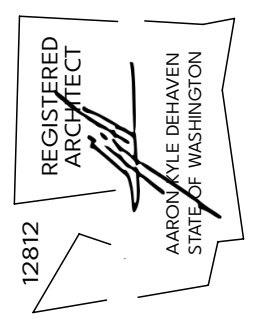
2 FROM SW
A020



3 FROM SE
A020



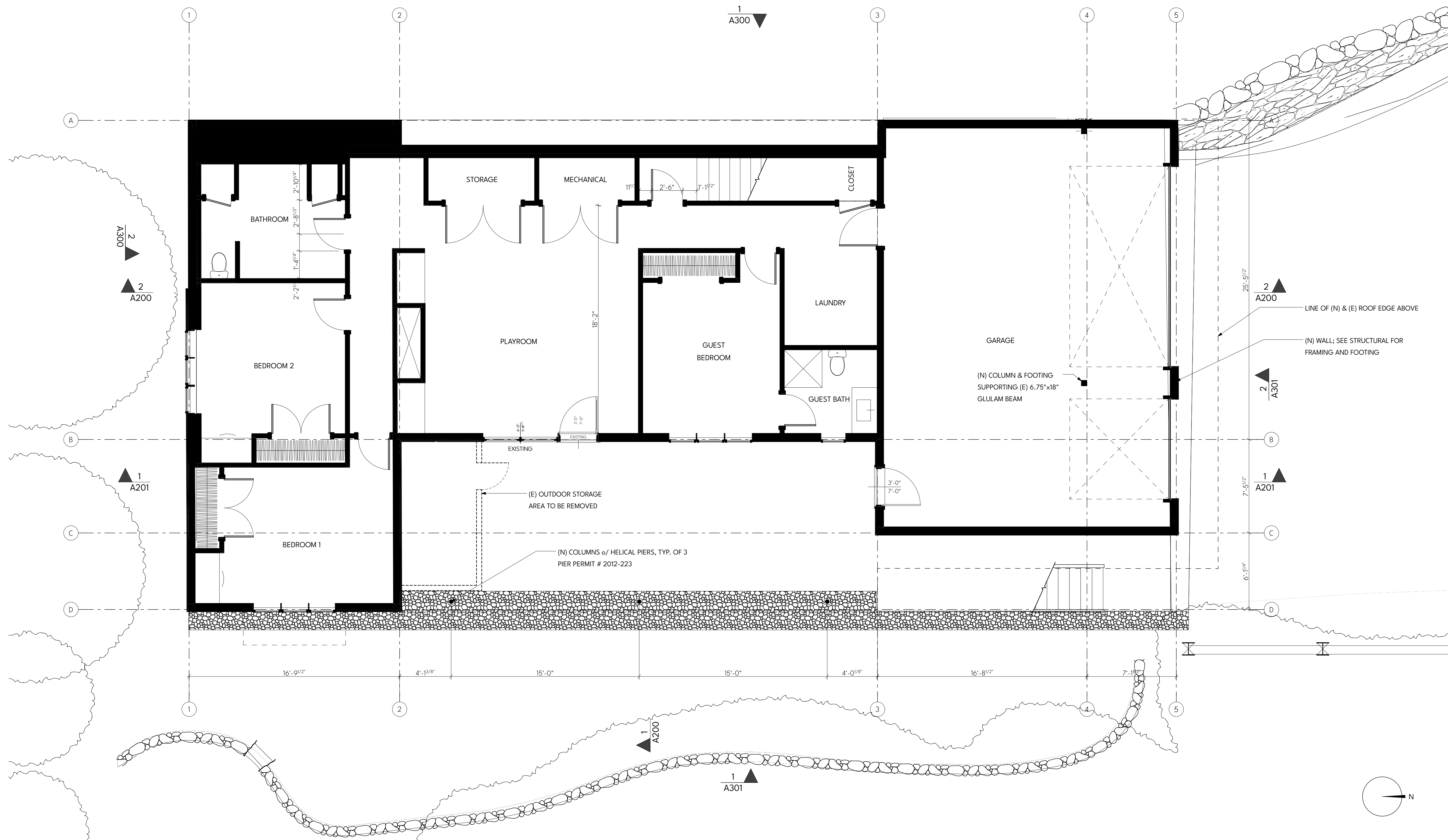
1 UPPER FLOOR PLAN
 A100 SCALE: 1/4" = 1'-0"



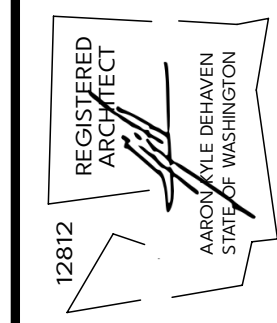
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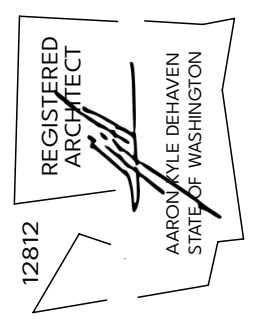
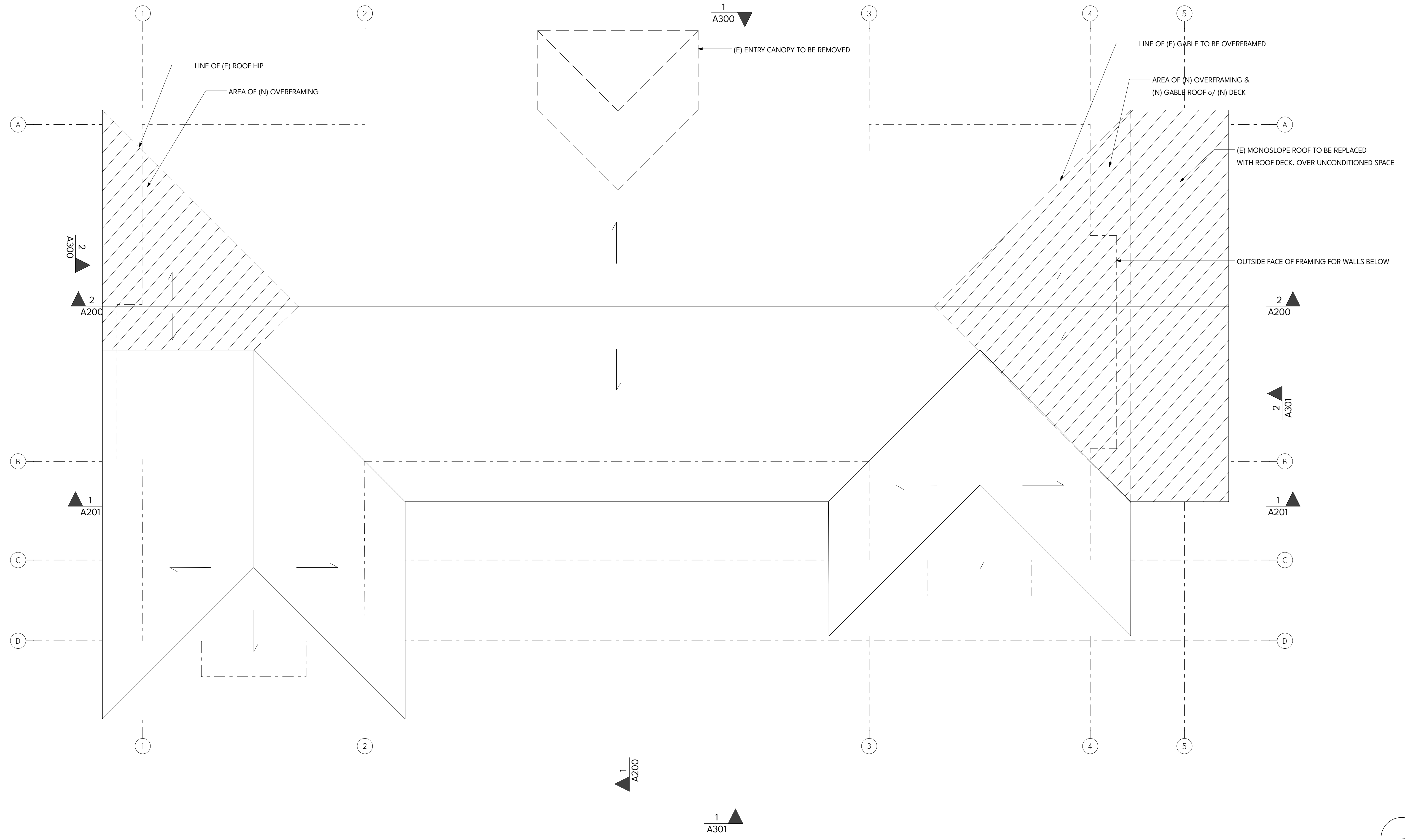
1 LOWER FLOOR PLAN
 A101 SCALE: 1/4" = 1'-0"



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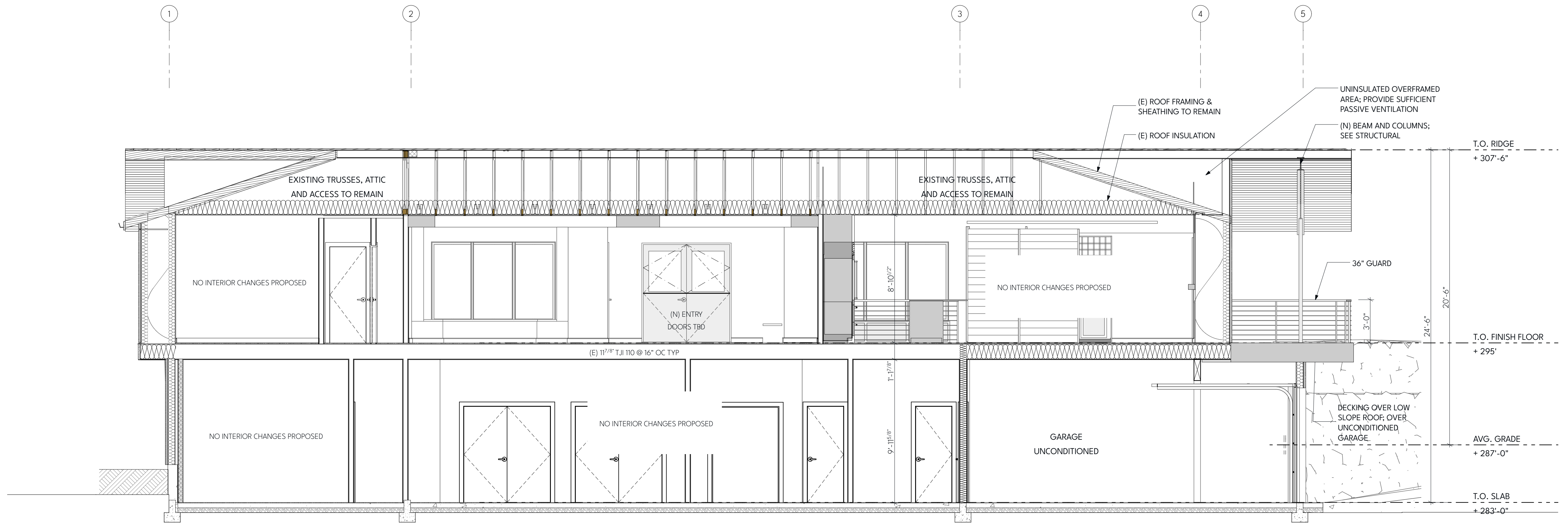
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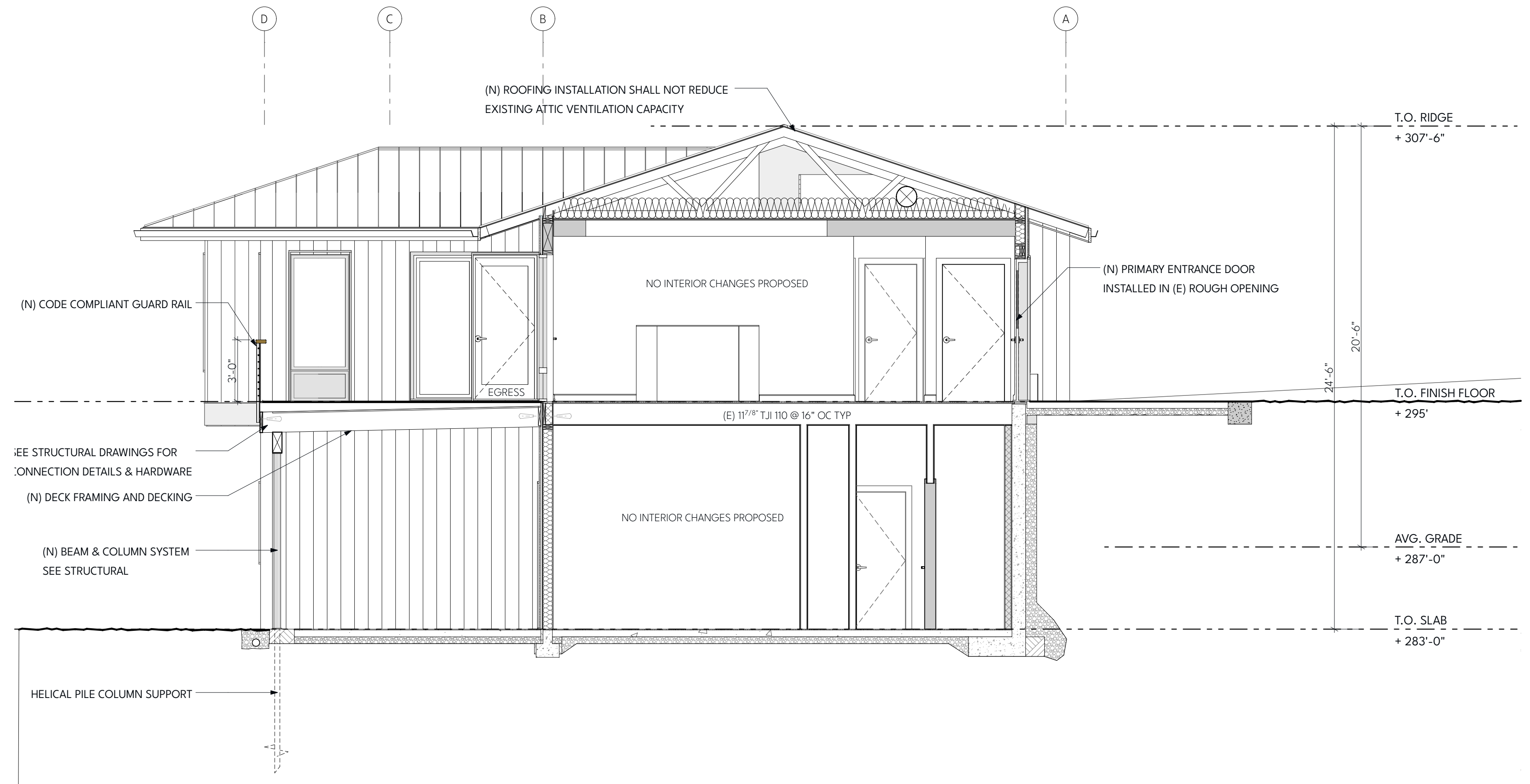
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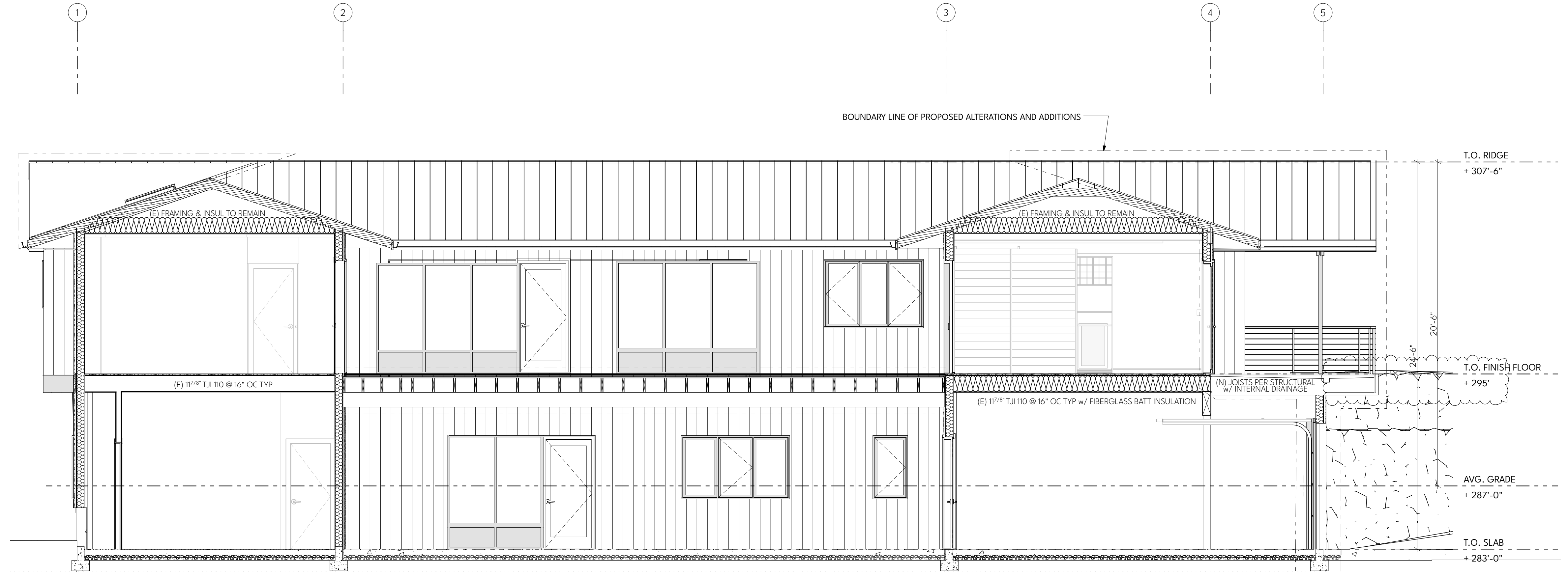
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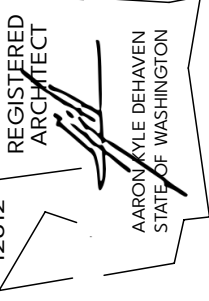
2 LONGITUDINAL AT RIDGELINE
A200 SCALE: 1/4" = 1'-0"



1 LATERAL AT ENTRY
A200 SCALE: 1/4" = 1'-0"



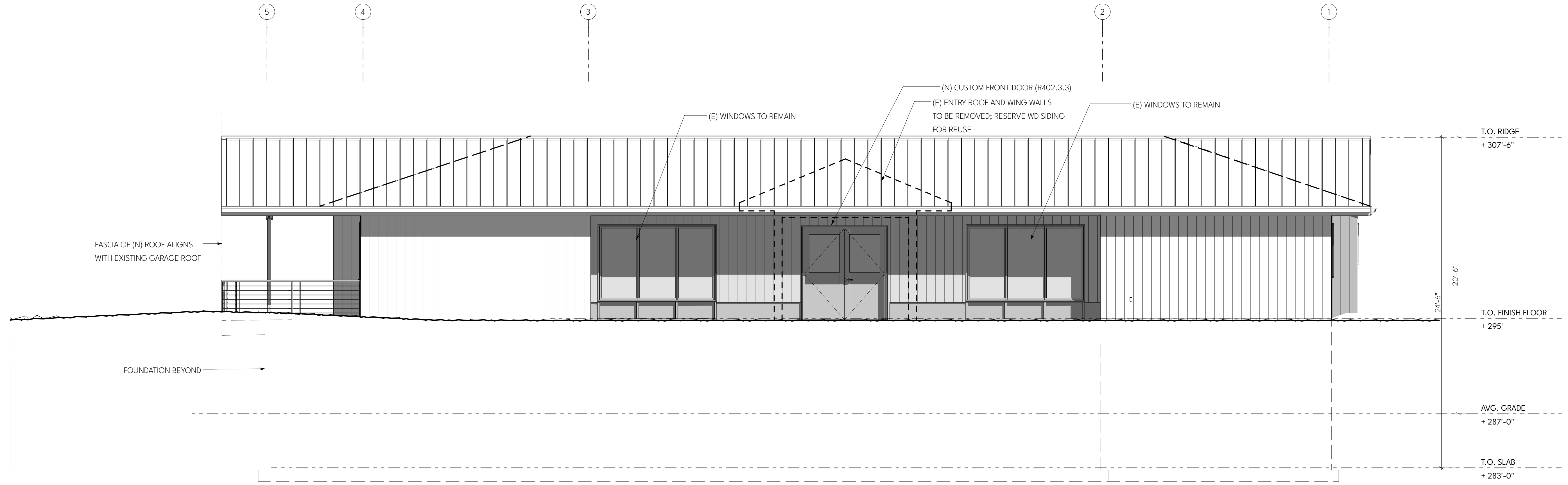
1 LONGITUDINAL THROUGH MASTER, DECK & BREAKFAST
 A201 SCALE: 1/4" = 1'-0"



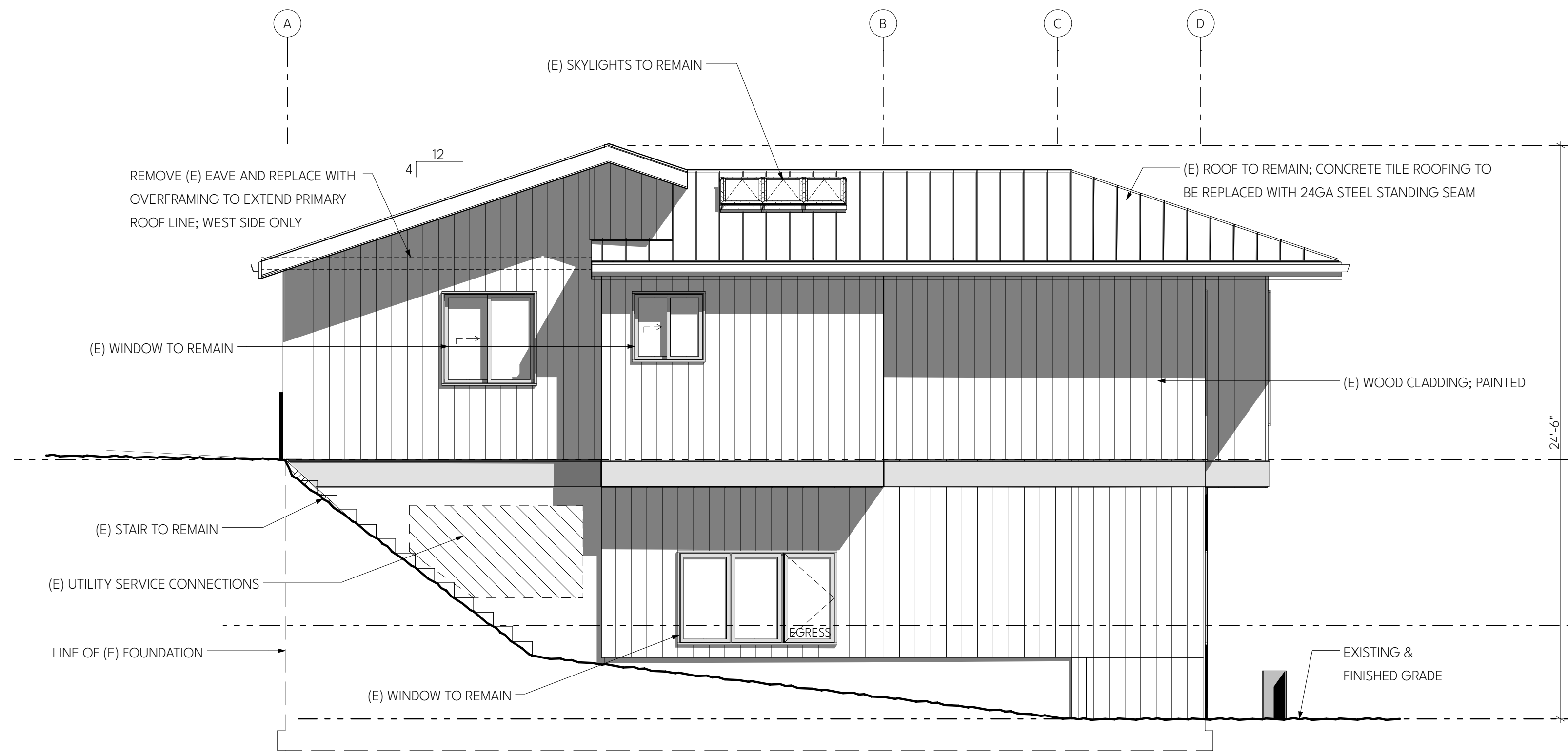
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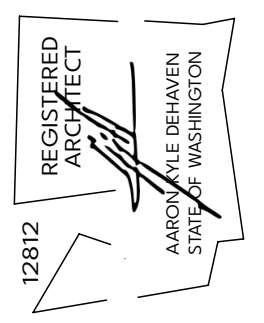
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1 WEST ELEVATION
A300 SCALE: 1/4" = 1'-0"



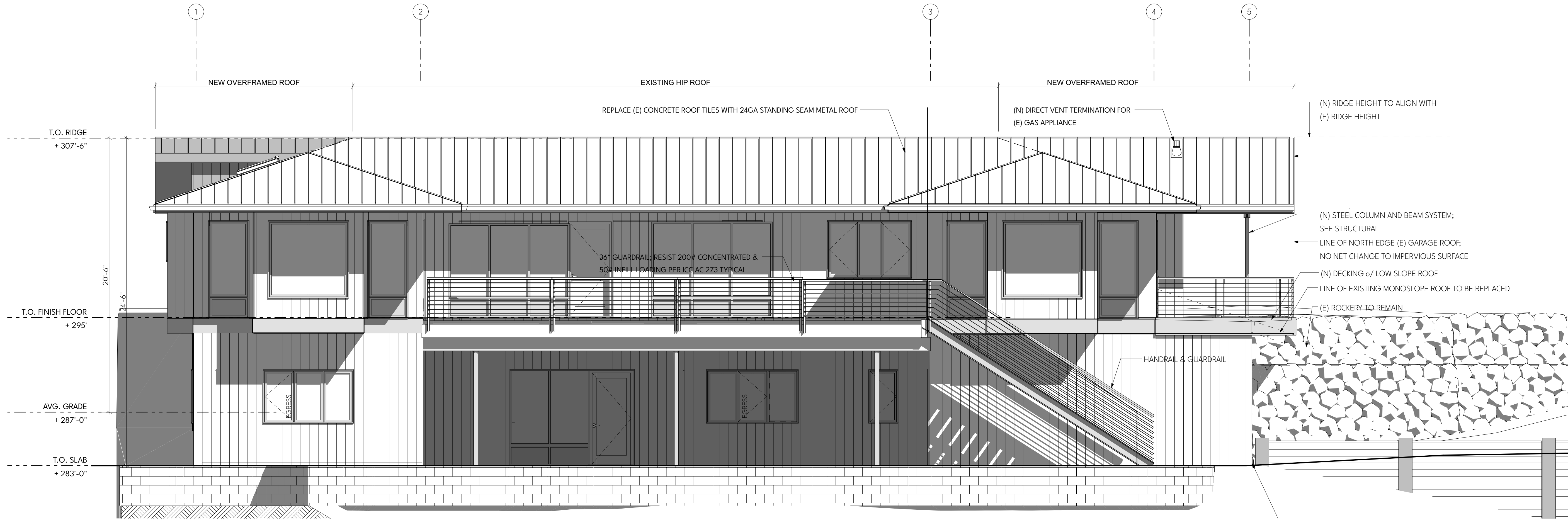
2 SOUTH ELEVATION
A300 SCALE: 1/4" = 1'-0"



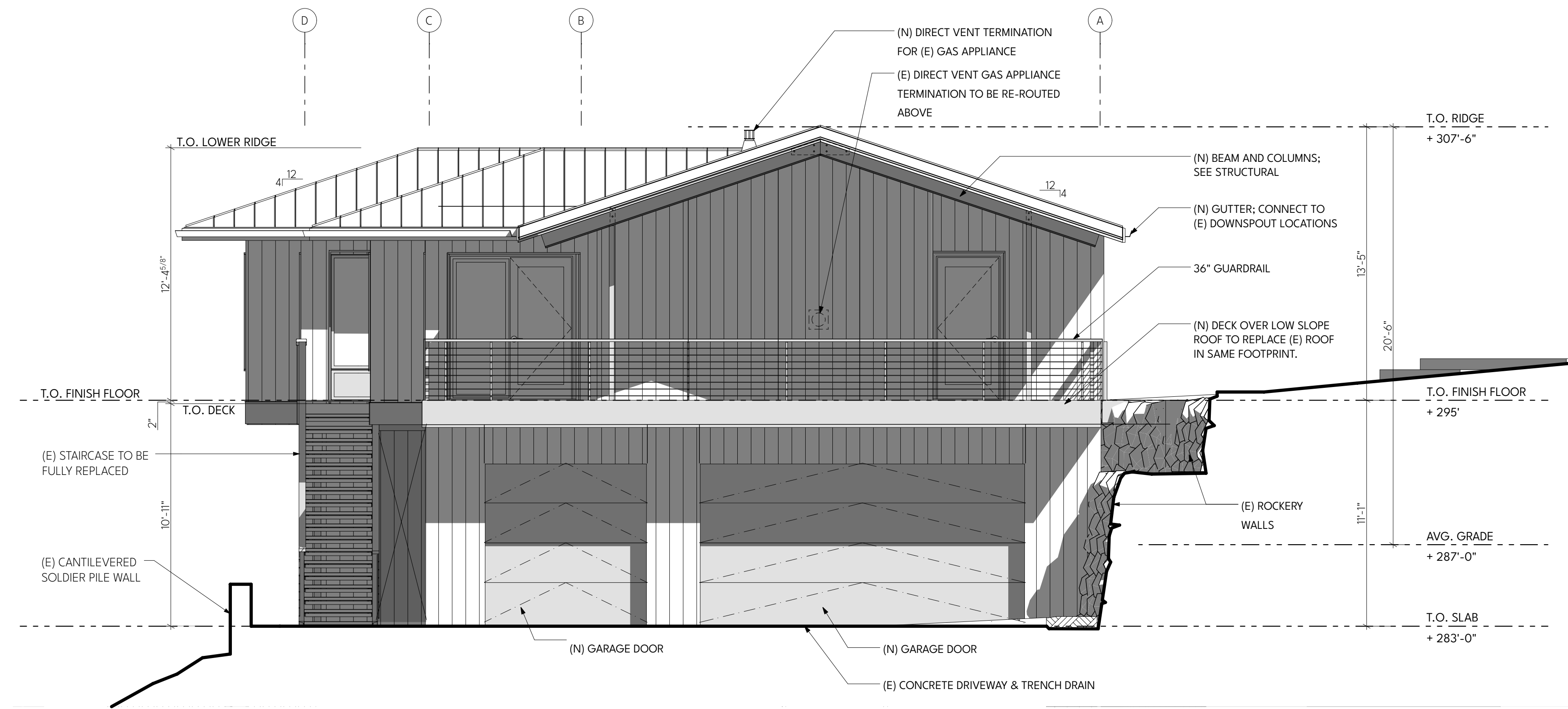
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1 EAST ELEVATION
A301 SCALE: 1/4" = 1'-0"

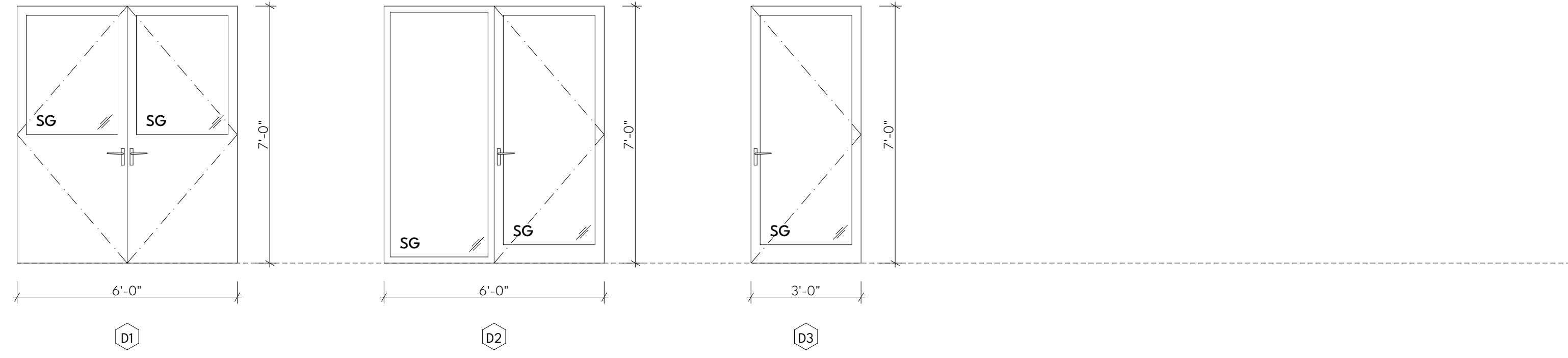


2 NORTH ELEVATION
A301 SCALE: 1/4" = 1'-0"

DOOR SCHEDULE

DOOR #	TYPE	WIDTH	HEIGHT	U-VALUE	NFRC	UA	MANUFACTURER
D1	DOUBLE SWING ENTRY	6'-0"	7'-0"	0.34 MAX	TBD	14.49 MAX	TBD
D2	SWING W/ SIDELITE	6'-0"	7'-0"	0.27	Y	11.34	MARVIN
D3	SINGLE SWING	3'-0"	7'-0"	0.27	Y	5.67	MARVIN
TOTAL:				0.30 MAX		31.5	

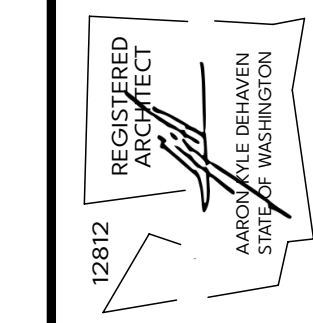
SG = SAFETY GLAZING



WINDOW SCHEDULE

WINDOW #	TYPE	WIDTH	HEIGHT	U-VALUE	NFRC	UA	MANUFACTURER
TOTAL:				0.30 MAX			

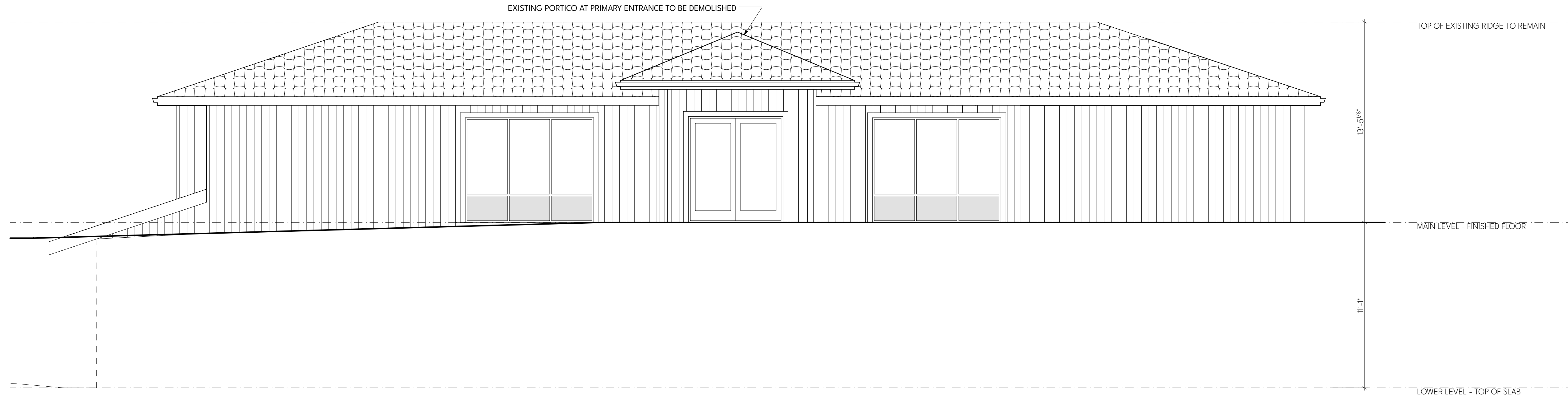
NO WINDOW MODIFICATIONS PROPOSED



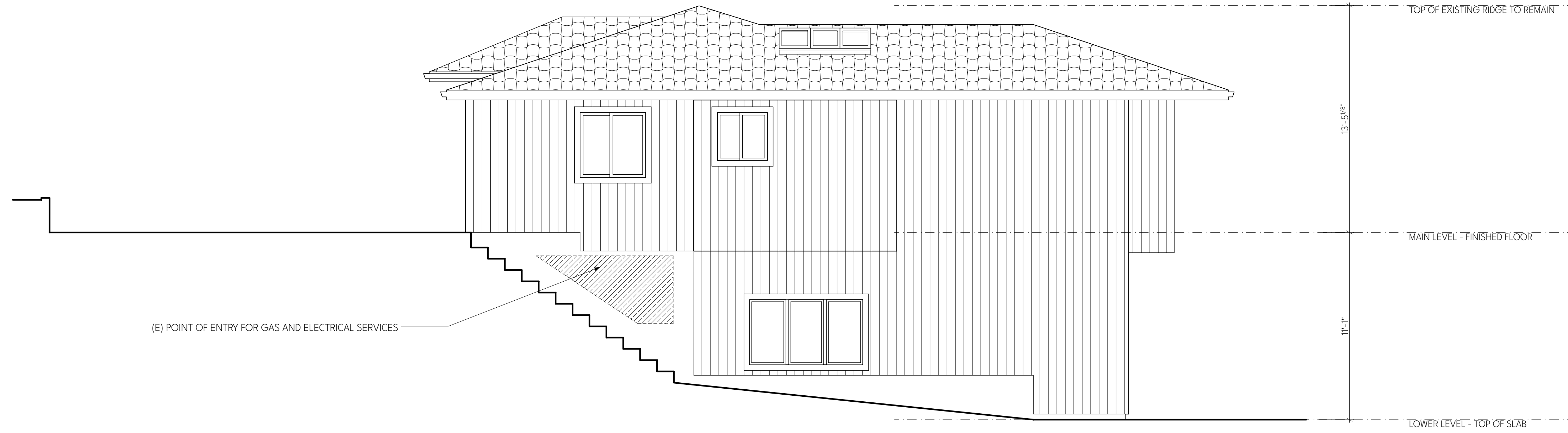
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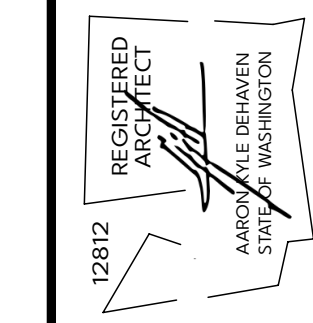
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1 EAST ELEVATION
A901 SCALE: 1/4" = 1'-0"



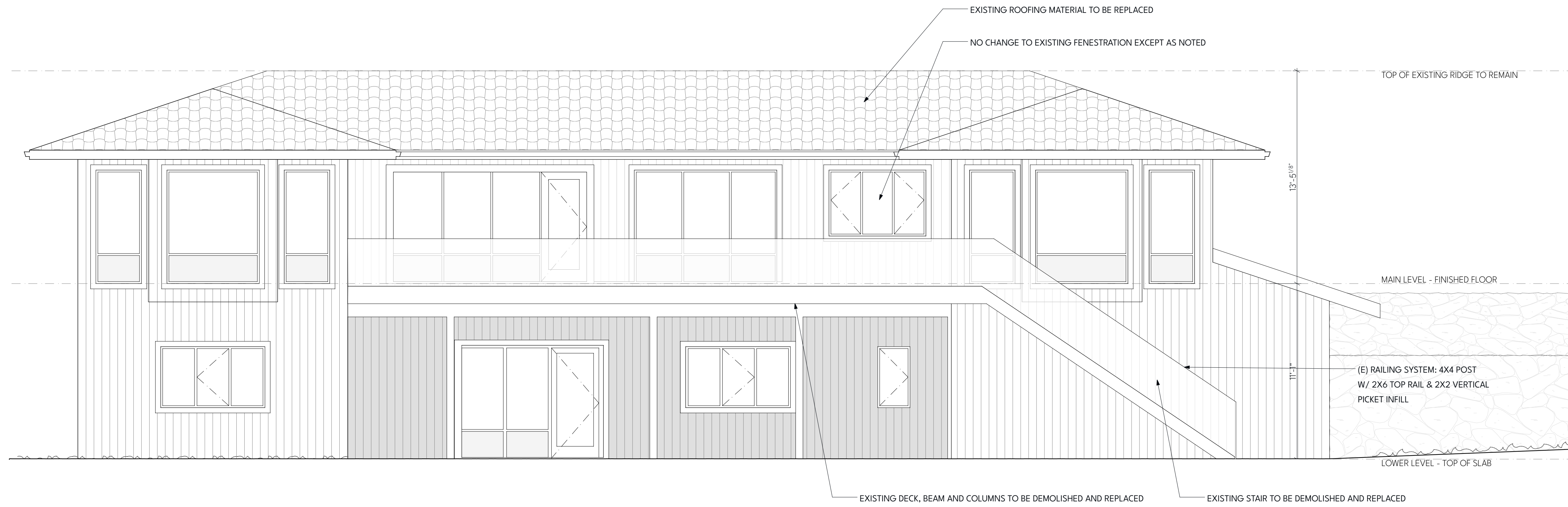
2 SOUTH ELEVATION
A901 SCALE: 1/4" = 1'-0"



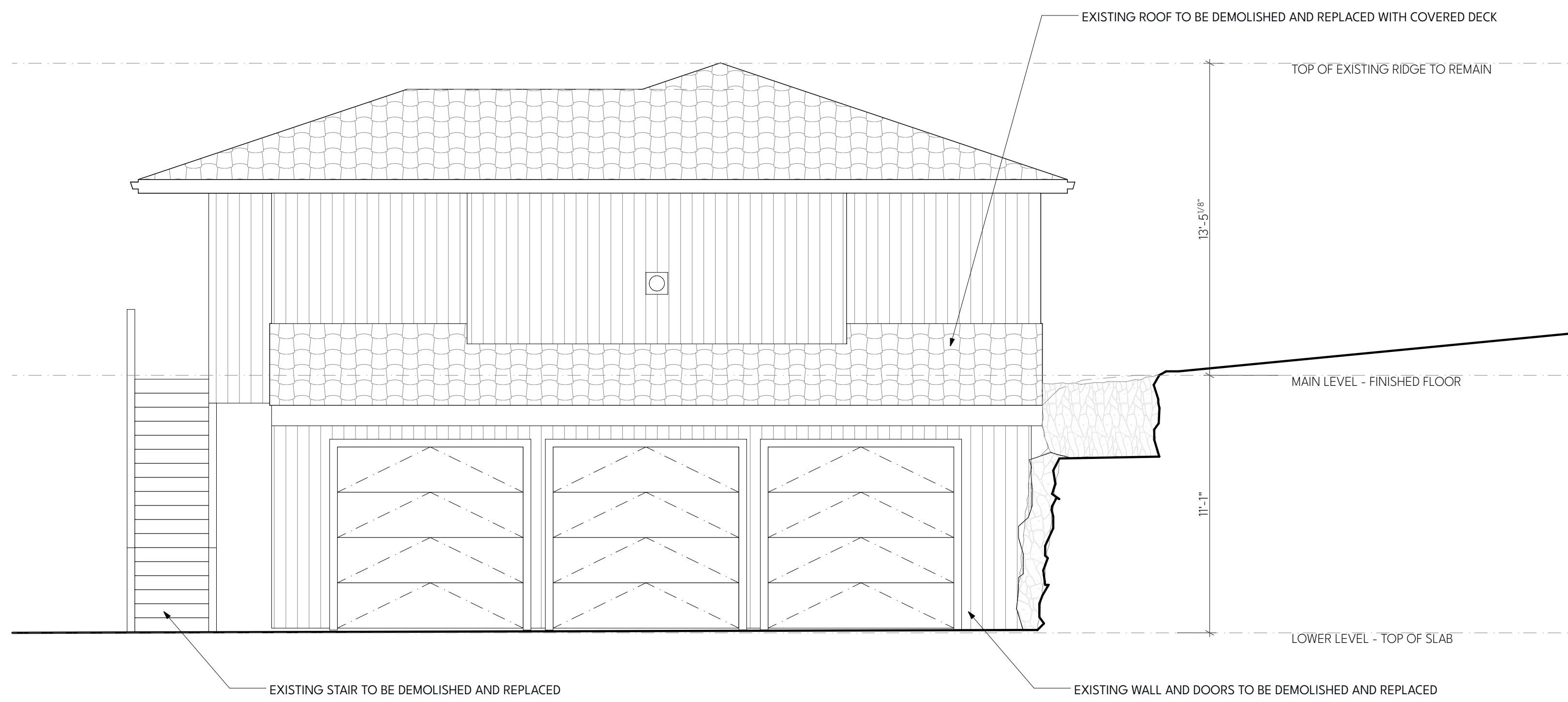
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1 WEST ELEVATION
A902 SCALE: 1/4" = 1'-0"



2 NORTH ELEVATION
A902 SCALE: 1/4" = 1'-0"

GENERAL NOTES

1.0 GENERAL

- 1.1 Construction shall conform to the 2018 INTERNATIONAL RESIDENTIAL CODE and all local regulatory agencies' requirements.
1.2 These drawings are the property of O.G. Engineering, PLLC ("Engineer").
1.3 Refer to Architectural Plans for all dimensions and elevations not shown.
1.4 The contractor shall be solely responsible for jobsite and construction safety and compliance with all current safety regulations.
1.5 Utility information is not shown on these drawings.
1.6 All waterproofing and drainage information shown on these drawings is for illustrative purposes only.
1.7 Review of shop drawings or other submittals by the Engineer is for general conformance with the contract documents and design concept only.

2.0 DESIGN BASIS - BUILDING STRUCTURES

- 2.1 Vertical Loads: Dead, Live, Snow, psf
2.2 Seismic Design Data (per the 2018 IBC): Risk Category: II, Importance Factor: Ie=1.0, Site Coordinates: 47.5814°N, 122.2390°W
2.3 Wind Design Data (per the 2018 IBC): Risk Category: II, Basic Wind Speed: 98 mph, Exposure Category: B, Topographic Factor: 1.6 (Per Mercer Island Wind Map)

3.0 INSPECTIONS

The construction work shall be inspected as required by the IRC Section R109. The contractor is solely responsible for understanding the requirements of and coordinating all inspections, observations and testing and ensuring that all required work is performed to the satisfaction of the inspector.

4.0 SHALLOW FOUNDATIONS

- 4.1 The following shallow foundation design criteria are assumed, have not been verified by a geotechnical engineer and therefore must be approved by the building official.
* Allowable Vertical Bearing Pressures: Dead + Live 2000 psf

4.2 Footing & Slab on Grade Excavations

Remove any deleterious, loose or softened material from footing & slab on grade excavations and compact sub-grades to a firm and unyielding condition. If loose sub-grades can not be adequately compacted, over-excavate loose material to competent soil and replace with properly compacted structural fill.

5.0 MATERIALS

5.1 Wood:

- 5.1.1 All untreated sawn lumber shall be Douglas Fir grade number 2, U.O.N. Mudsills and all sawn lumber in contact with concrete, masonry, ground, exposed to weather or moisture, shall be P.T. Hem Fir or Doug Fir grade number 2, U.O.N.
5.1.2 Glulam framing members shall be DF/DF, stress class 24F-1.8E, combination symbol 24F-V8, U.O.N.
5.1.3 All wood framing members shall have 19% maximum moisture content at time of installation.

5.2 Concrete:

Hardrock, normal-weight concrete with a minimum 28-day compressive strength of 3,000 psi for concrete exposed to weather and 2,500psi for concrete not exposed to weather. Slump range shall be 3-5 inches. Maximum aggregate size shall be 1".

5.3 Reinforcing Steel Bars:

ASTM A615, Grade 60

5.4 Welded Wire Fabric

ASTM A1064 or A185, Grade 75

5.5 Epoxy: (for dowels and anchors)

Concrete: Simpson SET-3G (Installed & inspected per ICC No. ESR-4057)

5.6 Bolts and Threaded Rods:

- 5.6.1 Threaded Rod: ASTM F1554 Grade 36
5.6.2 Sill Anchor Bolts: ASTM A307
5.6.3 Bolts in Timber Connections: ASTM A307

5.7 Structural Steel:

Plate and Bar: A36 (Fy = 36 ksi)
Channel (C or MC): A36 (Fy = 36 ksi)
Rectangular Tube (HSS): A500 Gr. B (Fy = 46 ksi)
Pipe (Pipe): A53 Gr. B (Fy = 35 ksi)

6.0 CONCRETE CONSTRUCTION

6.1 Concrete elements shall be constructed in single continuous pours, without construction joints, unless otherwise approved by the Engineer. Reinforcement shall be the longest lengths practical. Splices in rebar are not allowed in footings or walls less than 20 feet long. Lap splices shall be staggered at least 2 ft. in adjacent bars.

6.2 Reinforcement installation details, including rebar bends, hooks, splices and development lengths shall be in accordance with the requirements of the IRC Section R608.5.4, U.O.N. Concrete materials, forms, mixing and delivery shall be in accordance with the requirements of the IRC Section R404.1.3.3.

6.3 Slabs on Grade

6.3.1 Crack Control Joints

Cut crack control joints in top of slab @10'-0" o.c. (max.) each way. Joint depth shall be 1/4 of the slab depth or 1", whichever is greater. Joints shall be constructed of the early-entry saw-cut within 4 to 12 hrs of concrete placement, or early-entry saw-cut within 1 to 4 hrs of concrete placement.

6.3.2 Slab Sub-Base

Slab sub-base shall be 3/8" to 1/2" clean, crushed drain rock, compacted to a firm and unyielding condition.

6.4 Concrete Coverage over Reinforcing Steel

Unless otherwise noted, maintain the minimum concrete cover to face of reinforcement or anchors as follows:

- 1) 3" Where concrete is cast against and permanently exposed to earth except slab on grade.
2) 2" Where concrete is exposed to earth but formed, or exposed to weather.
3) 1 1/2" Where concrete is not exposed to earth or weather.

7.0 WOOD CONSTRUCTION

7.1 General Framing

Connections not specified on these drawings shall conform to the IRC fastening schedule, refer to Table R602.3(1). Depth of all posts in walls shall match stud depth, U.O.N. Block floor joist space solid under posts and cripple studs supporting headers and continue support to foundation. Face nail all plies of multi-ply studs with 10d@6" o.c. Obtain approval from engineer prior to ripping or creating notches or holes in framing members, U.O.N. Install double joists below all interior walls parallel to floor joists and solid blocking below all interior walls perpendicular to floor joists, U.O.N. All beams shall be continuous across supports unless explicitly shown as multiple pieces. Install full depth blocking between framing members over supports, unless otherwise noted. Orientation of dowels in exposed concealed framing connections (ex. "CBTZ") by architect, S.A.D.

7.2 Fasteners

Nails specified on these drawings are common nails, U.O.N. Fasteners in contact with P.T. wood, exposed to weather or in contact with ground shall be hot-dipped galvanized per IRC Section 317.3, or shall have equivalent corrosion resistance. Dissimilar metals & coatings shall not be in contact. Bolt holes shall be a minimum of 1/8" to a maximum of 1/4" larger than the bolt diameter. Bolts shall not be forcibly driven, and shall be tightened to the snug-tight condition. Install standard cut washers under all bolt heads and nuts bearing against wood.

7.3 Connectors

Connectors specified on these drawings are manufactured by the SIMPSON STRONG-TIE Company. Refer to latest catalog for information not specifically noted herein. Connectors in contact with P.T. wood, exposed to weather or in contact with ground shall be ZMAX or HDG galvanized. All connectors shall receive the maximum number of fasteners, U.O.N. Dissimilar metals & coatings shall not be in contact. Shim gaps in connectors for different framing sizes with plywood as required. Non-field-adjustable hangers specified as sloped or skewed shall be manufactured sloped or skewed.

7.4 Wood Structural Panels

WSPs shall bear the APA trademark and shall meet the requirements of the latest edition of USDCO PS1 or PS2. Use 10d common wire nails to fasten panels with 1 1/2" minimum penetration into framing at all panel edge and field nailing, U.O.N. Nails shall be located at least 3/8" from panel ends and edges. Stagger nails at adjoining panel edges. Drive nail heads flush with panel surface. Maintain 1/2" gap between all adjoining panel edges. Center interior panel joints on framing members or blocking. Provide 1/2" space between untreated panel and concrete or masonry. Minimum panel dimension shall be 2'-0". Panel storage and handling during transport and construction shall be in accordance with APA recommendations and shall protect the panels from prolonged exposure to moisture from rain, snow, ground or other sources. WSPs permanently exposed to weather shall be exterior grade.

7.5 Shear Walls and Exterior Wall Sheathing

7.5.1 Shear walls are noted on the plans. Shear walls shall be sheathed with 1/2" APA RATED SHEATHING, EXPOSURE 1 WSPs with a span rating of 3 1/2/6. Panels shall not be less than 4'-0"x8'-0", except at boundaries and changes in framing. Panels shall be laid with strength axis vertical. Install 2x blkg under all unsupported panel edges; all panel edges shall be supported by and fastened to min. 2x common studs or blocking, U.O.N. on shear wall schedule. Edge nail panels to posts that have holdowns or straps. Install double stud or min. 4x post at the ends of all shear walls. Provide solid blocking under double studs & posts between floors and continue support to foundation. See shear wall schedule for more information.

7.5.2 WSP Wall Nailing, U.O.N.:

Panel Edge Nailing: 10d@6" o.c. maximum.
Intermediate (Field) Nailing: 10d@12" o.c. maximum.

7.5.3 All new exterior walls not called out as shear walls shall be sheathed on their exterior face with 1/2" APA RATED SHEATHING, EXPOSURE 1 WSPs with a span rating of 3 1/2/6 and nailing per note 7.6.2., U.O.N. All other fasteners & requirements shall conform to the shear wall schedule for wall type (1).

7.6 Holdowns and Tiedown Straps

Holdowns and tiedown straps shall be attached to double studs or min. 4x posts, U.O.N. See latest Simpson Catalog for additional requirements not noted herein. See holdown schedule for anchor bolt sizes and additional specifications. Refer to note 7.1 for nailing and framing requirements at holdown/tiedown posts. Install solid post at shear wall corners or intersections where holdowns/tiedowns occur. All holdowns/tiedowns shall have the maximum number of fasteners.

7.7 Sill Anchor Bolts

There shall be a minimum of two sill anchor bolts per piece with one bolt located not more than 12" or less than 4 1/2" from each end of each piece. Holes in sills for bolts shall not be oversized. Sill anchor bolts shall be 3/8" dia with 7" min. embed. into concrete. Sill anchor bolts into existing concrete shall be all-thread rod, drill and epoxy. See shear wall schedule for spacing of sill anchor bolts in shear walls. Maximum sill anchor bolt spacing at non-shear-walls shall be 6'-0" o.c. at interior walls and 4'-0" o.c. at exterior walls. All sill anchor bolts at shear walls and mudsills shall be installed with 0.229"x3"x3" steel plate washers. Edge of sill anchor bolt plate washers shall be located 1/2" max. from inside face of wall sheathing or rim joist where occurs.

7.8 Floor and Roof Sheathing

7.8.1 Wood structural panel sheets at floors and roofs shall be laid with strength axis perpendicular to supports and continuous over two or more spans, unless otherwise noted on drawings. Stagger adjacent panels 4'-0" o.c. lengthwise.

7.8.2 Unless otherwise noted, typical roof sheathing shall be unblocked 5/8" APA RATED SHEATHING, EXPOSURE 1 WSPs with a span rating of 40/20. Panels shall be fastened to framing members with 10d nails @6" o.c. at all supported panel edges and 10d nails @12" o.c. intermediate (field) nailing. Install "PSC" sheathing clips (one mid-way between each support) at all unsupported panel joints.

7.8.3 Unless otherwise noted, typical floor sheathing shall be unblocked 3/4" APA RATED STURD-I-FLOOR EXPOSURE 1 WSPs with a span rating of 48/24 and T&G edges. Panels shall be fastened to framing members with 10d nails @6" o.c. at all supported panel edges and 10d nails @12" o.c. field nailing. Glue sheathing to all supports (including blocking) with 1/2" minimum beads of approved adhesive meeting APA specification AFG-01.

8.0 STRUCTURAL STEEL

8.1 Steel fabrication and erection shall be in accordance with "Specification for Structural Steel Buildings" (AISC 360-10).

8.2 Welding shall be in accordance with "Structural Welding Code - Steel" (AWS D1.1-10) Specifications. Minimum tensile strength of weld metal shall be 70 ksi, U.O.N. Welding electrodes shall be as recommended by their manufacturer for the position and other conditions of actual use. All welding shall be performed by AWS Certified Welders.

8.3 Bolt holes shall be drilled or punched. Bolt holes shall be standard, and hole size shall be 1/8" larger diameter than the nominal size of bolt used, U.O.N. Bolts shall be installed snug-tight, U.O.N.

8.4 All steel framing and fasteners exposed to weather or in contact with ground shall be hot-dipped galvanized after fabrication to meet the requirements of ASTM 153. Upon completion of erection; touch-up, de-slag, clean and apply zinc-rich primer to exposed welds or other unprotected markings incurred during the transportation, handling or erection process. Dissimilar metals & coatings shall not be in contact.

8.5 No penetrations shall be made through steel framing except as specifically indicated on these structural drawings or with the prior written permission of the engineer.

9.0 HELICAL PILES

9.1 Installation, testing and refusal criteria shall be in accordance with the project Geotechnical Report by Cobalt Geosciences dated 08/19/20. The Geotechnical Report is part of the construction documents and a copy may be obtained from the Geotechnical Engineer's office. The contractor shall notify Cobalt Geosciences (206-331-1097) in advance of any pile installation operations and Cobalt Geosciences should be present to observe and test, as necessary, the pile installation phases of the project.

9.2 Helical piles shall consist of 2 3/8" O.D. x0.203" (wall thickness) round shafts, ASTM A500, 80ksi steel. Helix plates shall be min. 3/8" thick x8", 10", 12" or 14" dia Gr. ASTM A572 Gr. 50 steel. Bolts shall be heavy hex ASTM A325 and nuts shall be heavy hex ASTM A194 Gr. 2 (not tempered). All helical pile components shall be H.D.G. Spacing of the lower helix on extensions shall be three times the diameter of upper helix on the preceding shaft.

9.3 Required Pile Capacity

The pile supplier shall select piles with a number, size and depth of helixes and extensions to provide the following minimum pile capacities and installation torques:

Minimum Installation Torque: 4000 ft-lbs
Required Ultimate Capacity: 36000 lbs
Required Allowable Capacity (SF=2): 18000 lbs

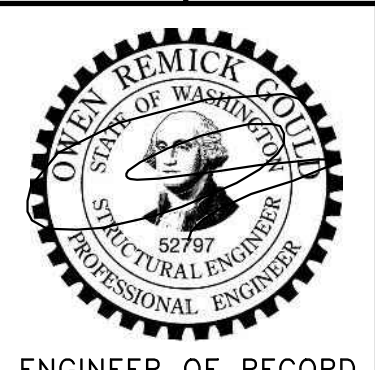
9.4 Submit shop drawings showing pile materials, construction details and demonstration of conformance with required pile capacity as noted above to Architect and Engineer for review and acceptance prior to fabrication.

ABBREVIATIONS

Table with 2 columns: Symbol and Description. Includes terms like ADJACENT, ARCHITECT, CONSTRUCTION JOINT, PERMIT SET, etc.

Table with 2 columns: REV and DATE. Includes PERMIT SET, 04-26-21.

ADDITIONS & ALTERATIONS
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Mercer Island, WA 98040
HAWN Architects
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Mercer Island, WA 98040

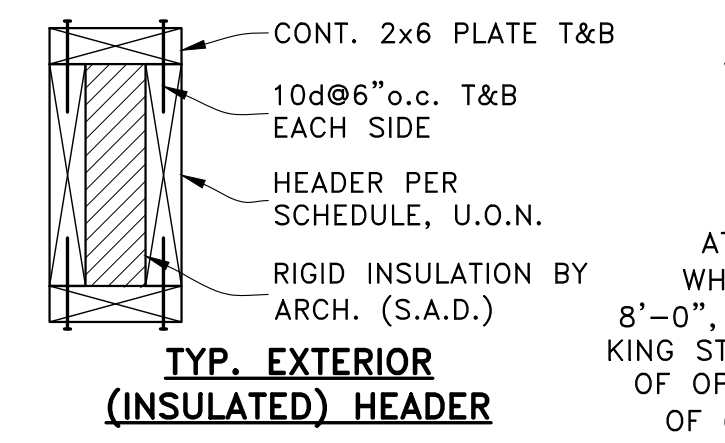


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Table with 2 columns: SCALE and SHEET NO. Includes AS NOTED, 20025, SHEET NO. S1.

EXTERIOR HEADER SCHEDULE, U.O.N.

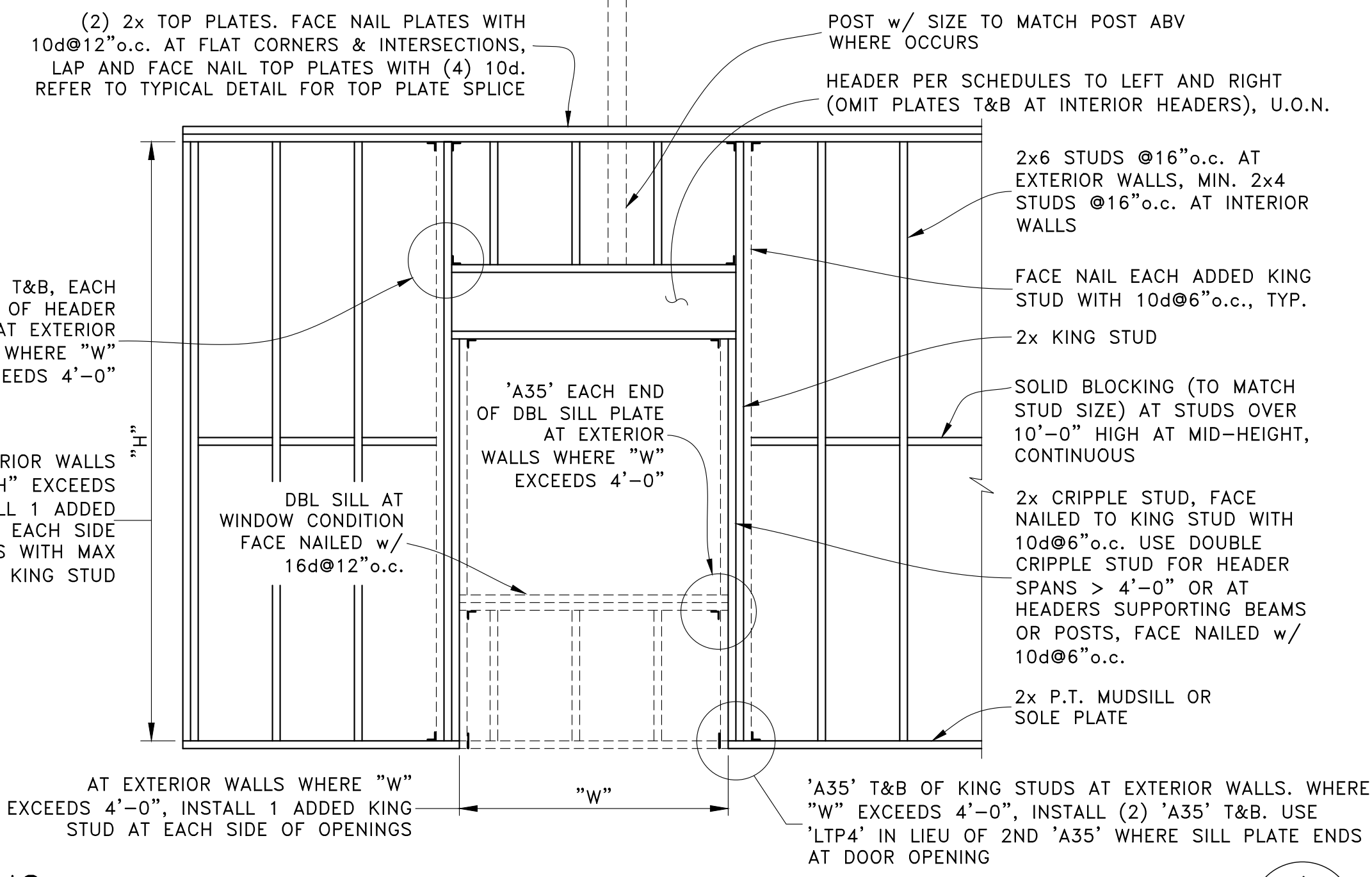
"W" MAX. OPENING	MIN. HEADER
4'-0"	2-2x8
6'-0"	2-2x10
8'-0"	2-2x12
10'-0"	2-2x14



INTERIOR HEADER SCHEDULE, U.O.N.

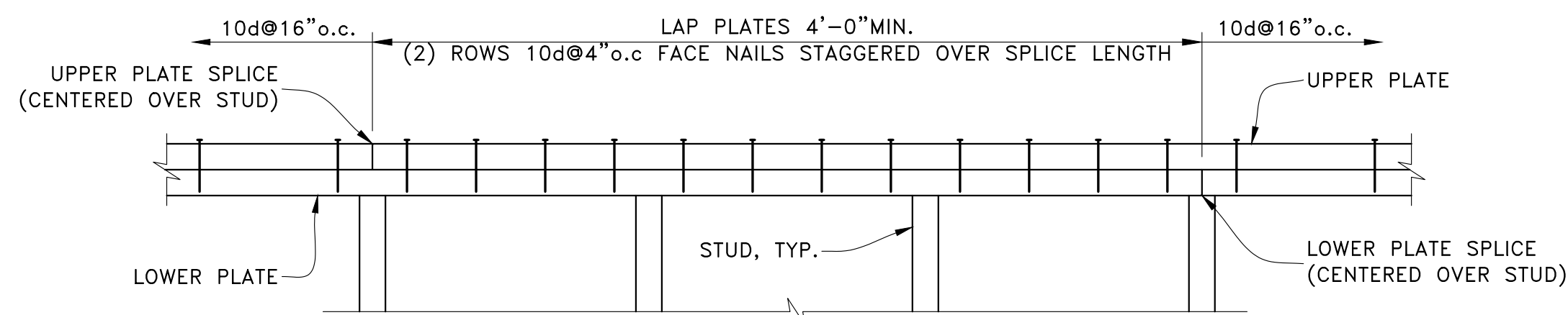
"W" MAX. OPENING	MIN. HEADER
4'-0"	4x8
6'-0"	4x10
8'-0"	4x12
10'-0"	4x14

NOTE: AT 4x INSULATED HEADER, PLACE 2" INSULATION ON INTERIOR SIDE



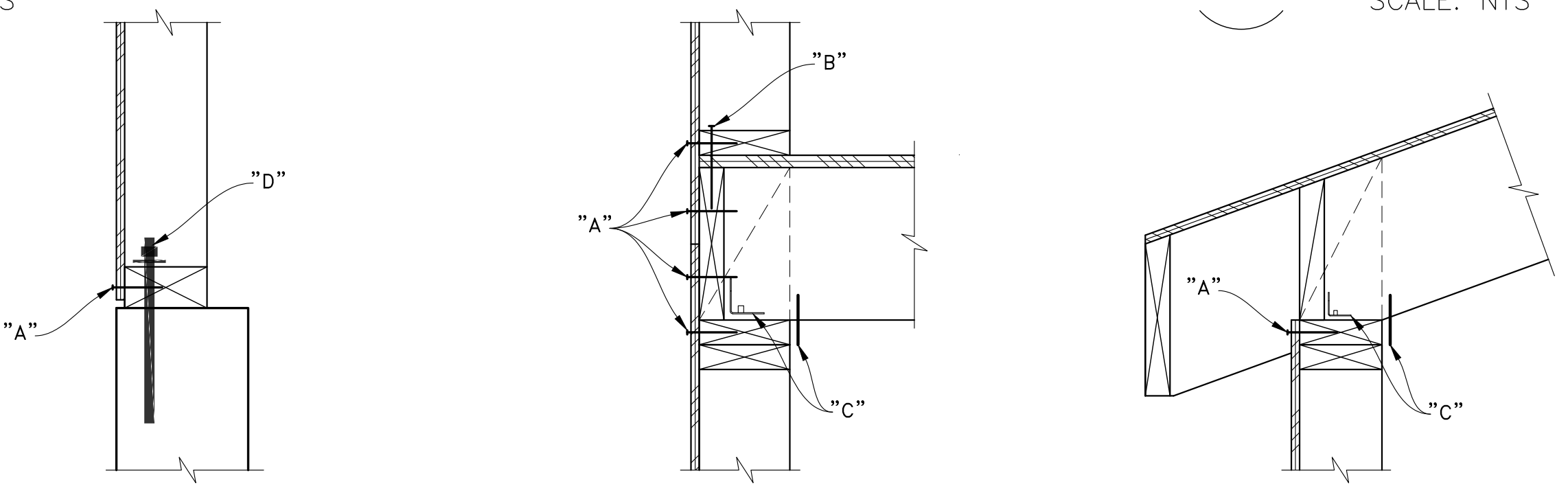
TYPICAL STUD WALL FRAMING

SCALE: NTS



TYPICAL DOUBLE TOP PLATE SPLICE

SCALE: NTS



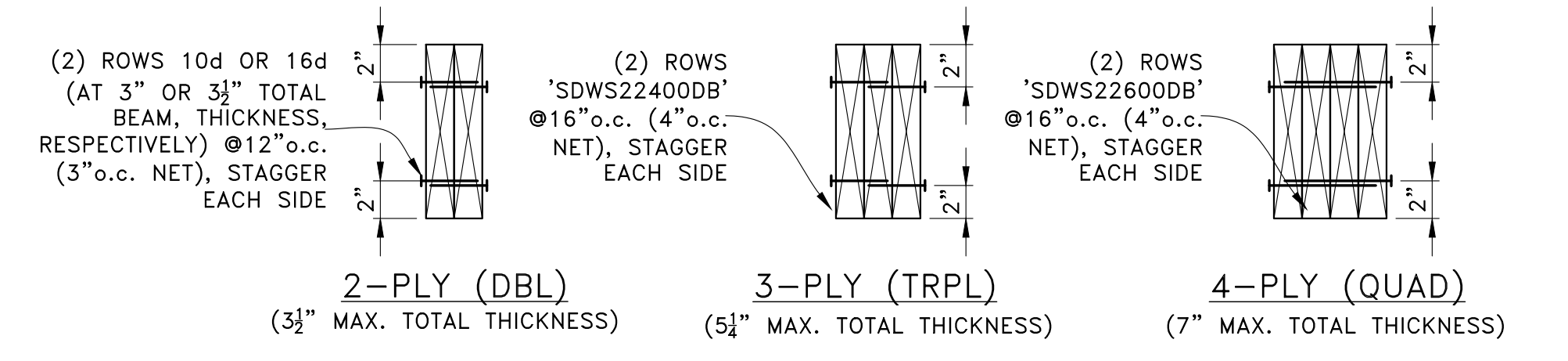
SHEAR WALL SCHEDULE (1/2" SHEATHING-RATED WOOD STRUCTURAL PANELS)

SHEAR WALL MARK	CAPACITY (PLF)	EDGE NAILING "A"	FIELD NAILING "B"	FRAMING AT ADJOINING PANEL EDGES	SOLE PLATE FASTENERS "B"	FRAMING CLIPS "C"	SILL ANCHOR BOLT SPACING - "D"
①	310	10d@6" o.c.	10d@12" o.c.	2x NOMINAL	'SDS25600' @ 8" o.c. ⁴	'A34' OR 'LTP4' @ 16" o.c. ⁵	4'-0" o.c. ⁶
②	460	10d@4" o.c.	10d@12" o.c.	2x NOMINAL	'SDS25600' @ 8" o.c. ⁴	'A34' OR 'LTP4' @ 8" o.c. ⁵	2'-8" o.c. ⁶
③	600	10d@3" o.c. ¹	10d@12" o.c.	3x OR 2-2x NOMINAL ³	'SDS25600' @ 8" o.c. ⁴	'A34' OR 'LTP4' @ 8" o.c. ⁵	2'-8" o.c. ⁶
④	770	10d@2" o.c. ¹	10d@12" o.c.	3x OR 2-2x NOMINAL ³	'SDS25600' @ 4" o.c. ⁴	'A34' OR 'LTP4' @ 8" o.c. ⁵	1'-4" o.c. ⁶
DBL SIDED ②	920	10d@4" o.c. ¹	10d@12" o.c.	3x OR 2-2x NOMINAL ³	'SDS25600' @ 4" o.c. ⁴	'A34' OR 'LTP4' @ 4" o.c. ⁵	1'-4" o.c. ⁶
DBL SIDED ③	1200	10d@3" o.c. ¹	10d@12" o.c.	3x OR 2-2x NOMINAL ³	'SDS25600' @ 4" o.c. ⁴	'A34' OR 'LTP4' @ 4" o.c. ⁵	1'-4" o.c. ⁶
DBL SIDED ④	1540	10d@2" o.c. ¹	10d@12" o.c.	3x OR 2-2x NOMINAL ³	'SDS25600' @ 3" o.c. ⁴	'A34' OR 'LTP4' @ 4" o.c. ⁵	8" o.c. ⁶

- NOTES**
- 1) STAGGER ROWS OF EDGE NAILING 1/2" APART. ON DBL SIDED WALLS, STAGGER EDGE NAILS ON PANELS ON OPPOSITE SIDES OF WALL.
 - 2) NAILING TO ALL INTERMEDIATE FRAMING MEMBERS IN FIELD OF PANEL
 - 3) PANEL EDGE NAILING SHALL BE STAGGERED. 2-2x FRAMING MEMBERS SUPPORTING PANEL EDGES SHALL BE FACE NAILED WITH 10d, SPACING TO MATCH PANEL EDGE NAILING, STAGGERED. STAGGER PANEL EDGES IN OPPOSITE PANELS MIN. 2'-0" APART ON DBL SIDED SHEAR WALLS.
 - 4) SCREWS SHALL HAVE MIN. 2" PENETRATION INTO RIM JOIST/ BLOCKING - USE LONGER SCREWS IF NECESSARY.
 - 5) FRAMING CLIPS ARE ONLY REQUIRED WHERE SPECIFIED ON FRAMING DETAILS.
 - 6) SEE GENERAL NOTES 7.6 & 7.8 FOR MORE INFORMATION.

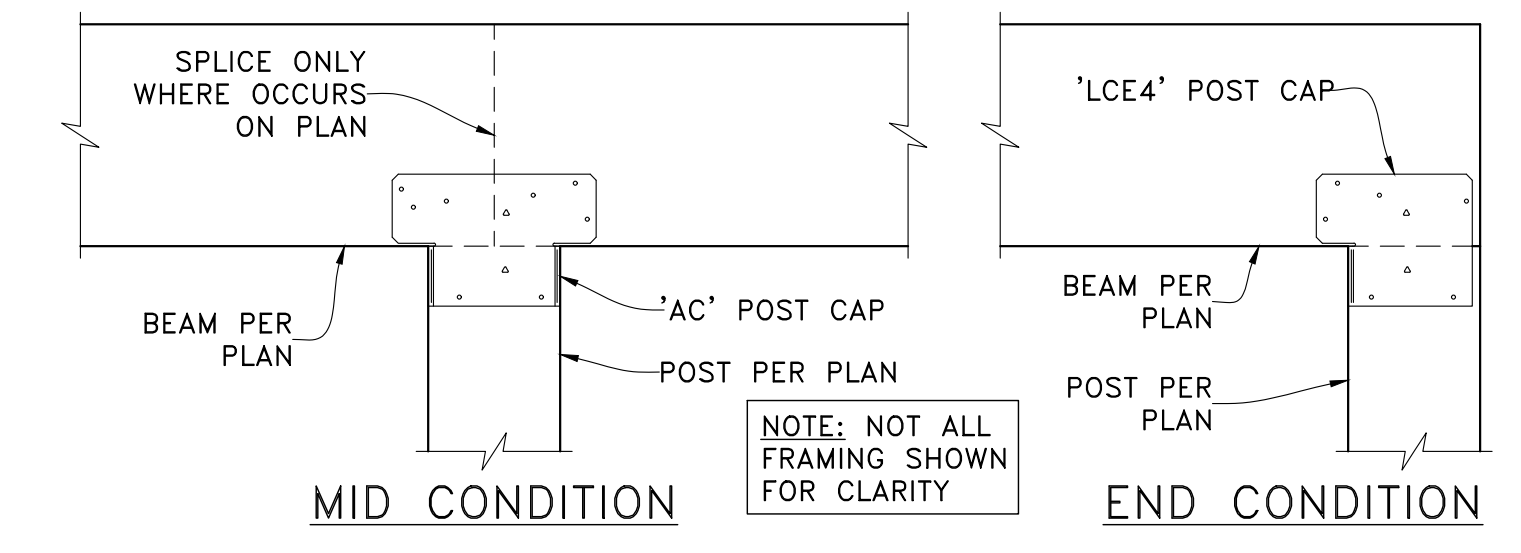
SHEAR WALL SCHEDULE (S.W.S.)

SCALE: NTS



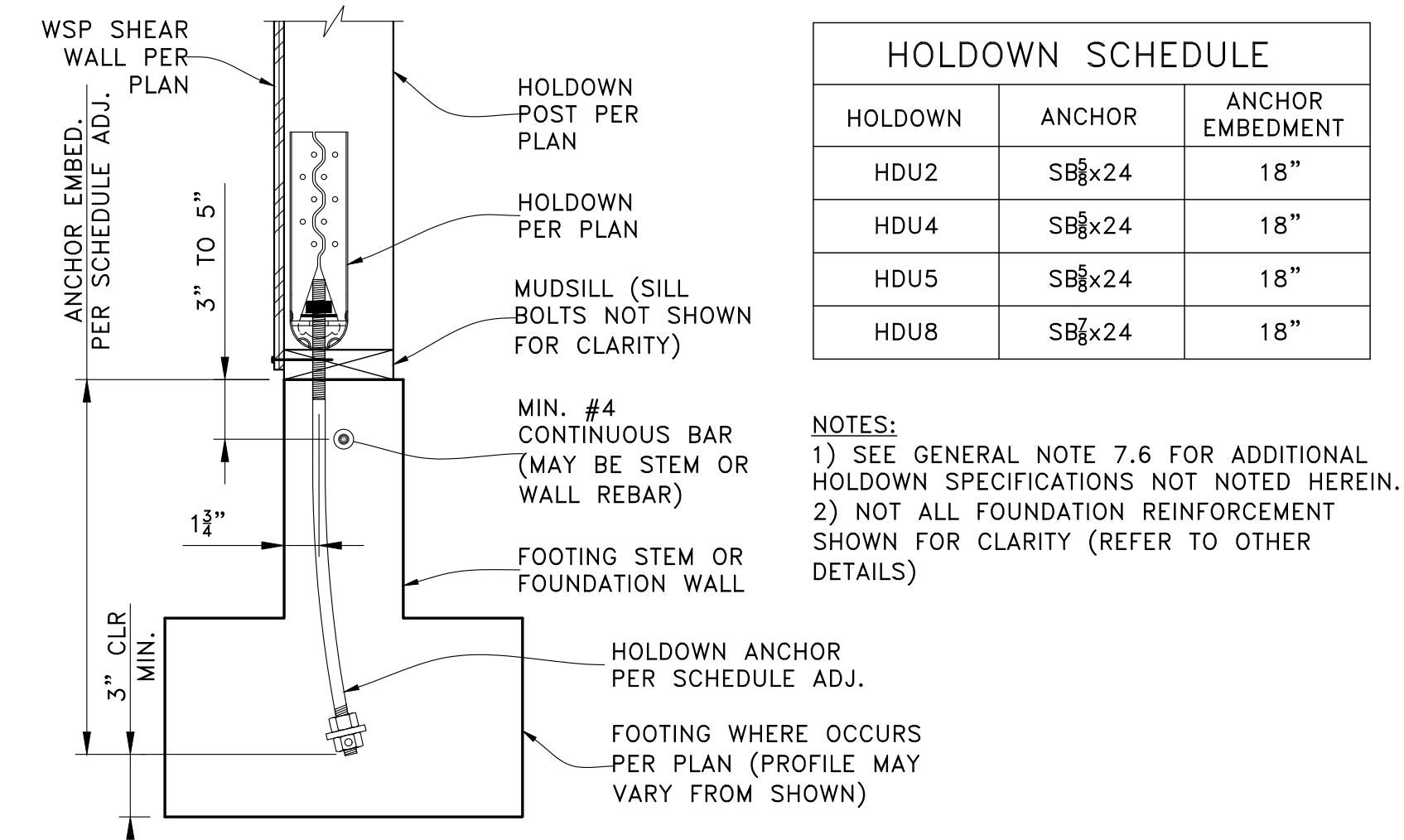
TYPICAL MULTI-PLY BEAM FASTENING

SCALE: NTS



BEAM TO ISOLATED POST

SCALE: NTS



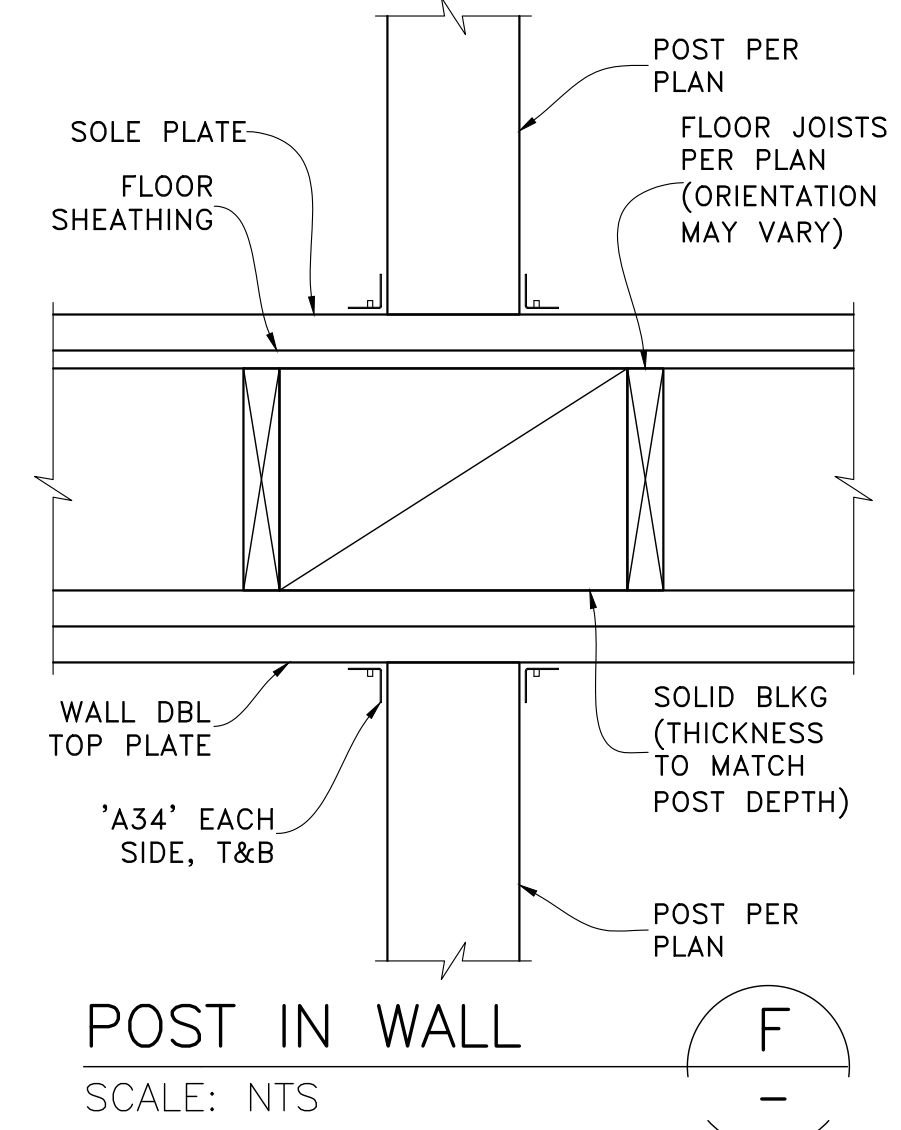
TYPICAL HOLDDOWN AT FOUNDATION

SCALE: NTS

HOLDOWN SCHEDULE

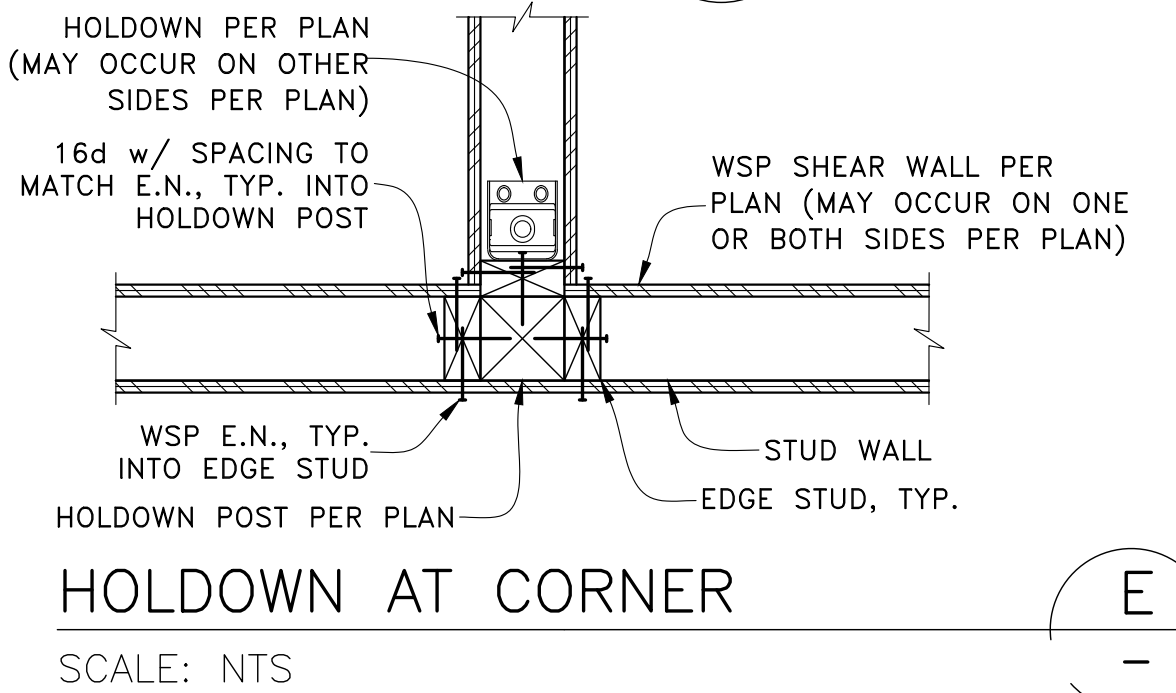
HOLDOWN	ANCHOR	ANCHOR EMBEDMENT
HDU2	SB8x24	18"
HDU4	SB8x24	18"
HDU5	SB8x24	18"
HDU8	SB8x24	18"

- NOTES:**
- 1) SEE GENERAL NOTE 7.6 FOR ADDITIONAL HOLDOWN SPECIFICATIONS NOT NOTED HEREIN.
 - 2) NOT ALL FOUNDATION REINFORCEMENT SHOWN FOR CLARITY (REFER TO OTHER DETAILS)



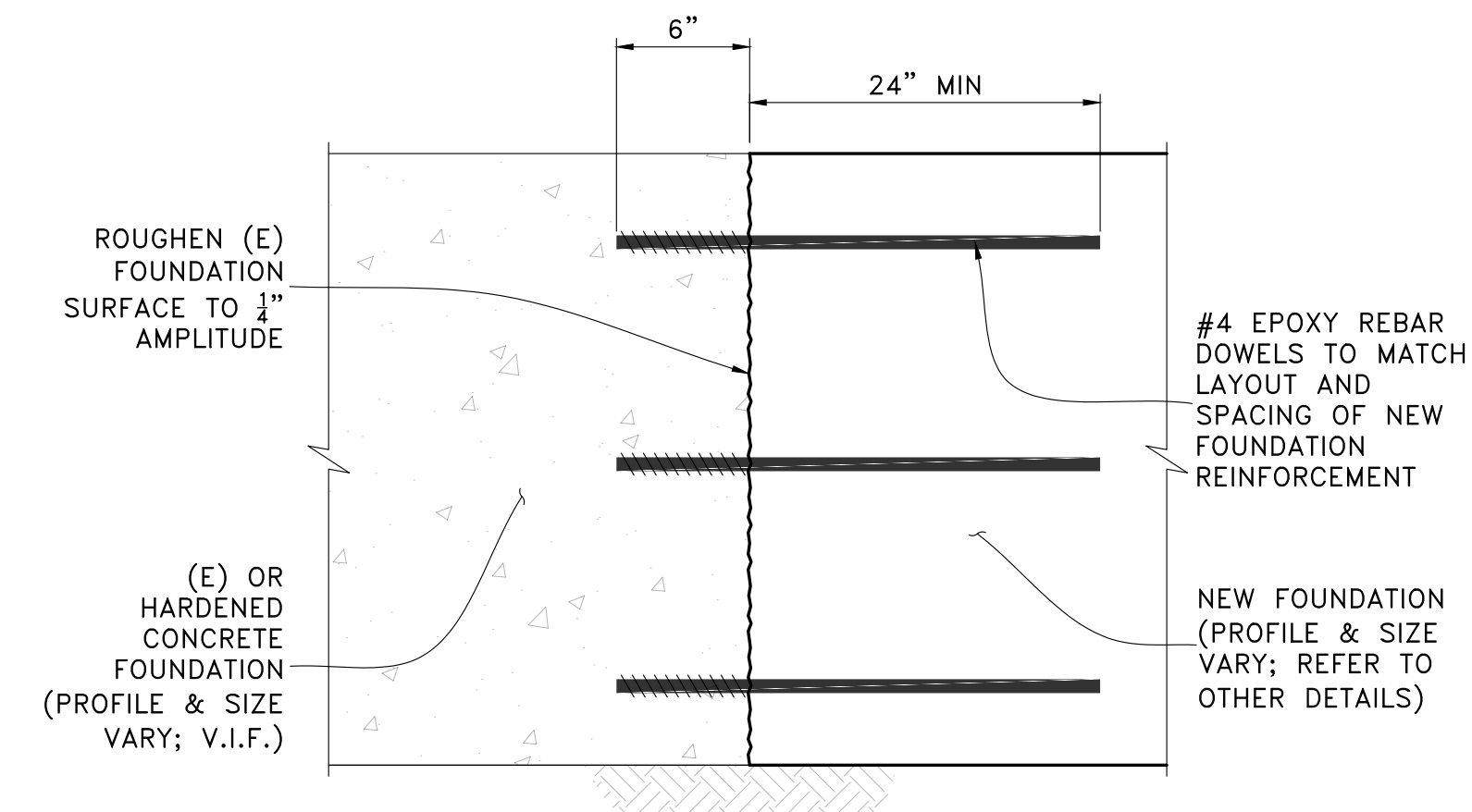
POST IN WALL

SCALE: NTS



HOLDDOWN AT CORNER

SCALE: NTS



TYPICAL NEW TO EXISTING FOUNDATION

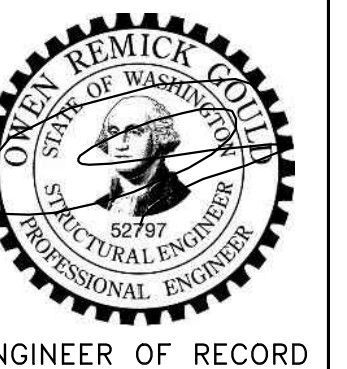
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PERMIT SET

REV	DATE	DESCRIPTION
04-26-21		PERMIT SET

ADDITIONS & ALTERATIONS
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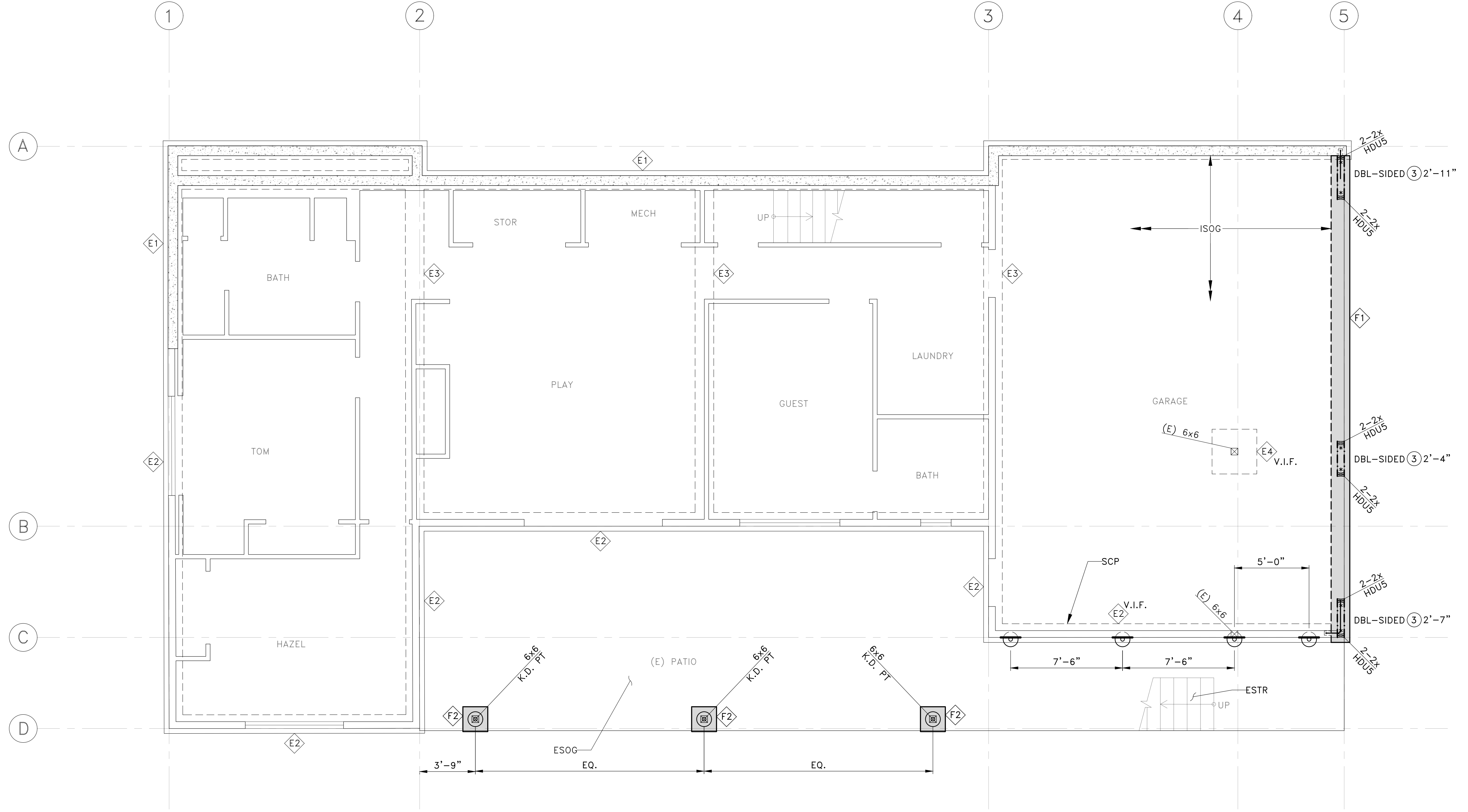
TYPICAL DETAILS

PLAN LEGEND

	SHALLOW CONCRETE SPREAD FOOTING PER FOUNDATION SCHEDULE BLW
	(E) 8" CONCRETE FOUNDATION WALL
	(E) CONCRETE SPREAD FOOTING PER FOUNDATION SCHEDULE BLW
	NEW OR EXISTING STUD WALL ABOVE FLOOR
	1/2" W.S.P. SHEAR WALL TYPE (X) w/ MIN. LENGTH 'L', PER (H) (A) (K)
	POST ABOVE FLOOR PER (F) (C)
	POST & HOLDOWN PER (K)
	EPOXY REBAR DOWEL NEW (J) BEND AROUND CORNERS TO (E) FOUNDATION PER (S2) WHERE OCCURS
	RETROFIT HELICAL PILE w/ WALL BRACKET CENTERED BLW POST, U.O.N., (B) 7 LOCNS PER SHEET S1 GENERAL NOTE 9.0 AND (S6)
SCP	LIFTING OF (E) EAST GARAGE FOUNDATION USING HELICAL PILES TO CORRECT SETTLEMENT BY OTHERS
ESTR	EXTERIOR STAIR PER (F) (S6)
ESOG	(E) PATIO CONCRETE SLAB ON GRADE (4" MIN, V.I.F.)
ISOG	(E) PATIO CONCRETE SLAB ON GRADE (4" MIN, V.I.F.)

FOUNDATION SCHEDULE

F1	15" WIDE EXTERIOR STRIP FOOTING PER (A) (S6)
F2	18" SQ. PILE CAP w/ HELICAL PILE PER SHEET S1, GENERAL NOTE 9.0 & (C) (S6) 3 LOCNS
E1	(E) 8" FOUNDATION WALL w/ 15" WIDE T-FOOTING
E2	(E) 15" WIDE EXTERIOR STRIP FOOTING (V.I.F. WHERE NOTED ON PLAN)
E3	(E) 15" WIDE INTERIOR STRIP FOOTING
E4	(E) 3'-0" SQ. INTERIOR PAD FOOTING (MIN, V.I.F.)



PERMIT SET	
REV	DATE
04-26-21	PERMIT SET

PROJECT: ADDITIONS & ALTERATIONS
 3206 74th Pl SE
 Mercer Island, WA 98040

TEL: HAVN Architects
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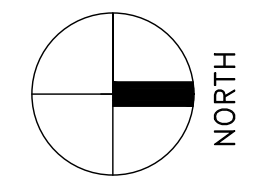


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SHEET TITLE: LOWER FLOOR FRAMING & FOUNDATION PLAN

SCALE: AS NOTED	SHEET NO. S3
JOB NO. 20025	

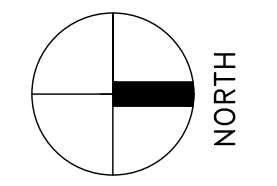


PLAN LEGEND

	NEW OR EXISTING STUD WALL ABOVE FLOOR
	NEW OR EXISTING WALL BELOW FLOOR
	NEW OR EXISTING WINDOW BY ARCH (S.A.D.)
	3/4" W.S.P. SHEAR WALL TYPE (X) w/ MIN. LENGTH 'L', PER (H/S2) (G/S6) (K/S6)
	POST ABOVE OR BELOW FLOOR PER (F-G/S2) U.O.N.
	POST & BENT TIEDOWN STRAP PER PLAN AND SIMPSON MANUAL
	METAL STRAP PER PLAN
DST1	CONT. 'CS16' STRAP o/ DECK SHEATHING o/ 2x BLKG PER (K/S6)
DST2	CONT. 'CS16' STRAP o/ INTERIOR & EXTERIOR WALL SHEATHING o/ 2x4 FLAT BLKG, CONTINUE FOR 2'-0" MIN ACROSS OUTSIDE FACE OF HEADERS OVER GARAGE DOORS
PBX	SUSPENDED PLANTER BOX PER (E/S7)
EHDR	(E) DROPPED HEADER OVER WALL OPENING BLW
ISTR	(E) INTERIOR STAIR
BEAM HANGER	FLUSH-FRAMED JOIST OR BEAM CONNECTION; SEE FRAMING SCHEDULE FOR HANGERS, U.O.N. ON PLAN OR DETAILS (JOIST HANGERS NOT SHOWN ON PLAN FOR CLARITY)
	JOIST OR BEAM BEARING ON DROPPED BEAM OR HEADER (BEARING WALL SIM). POST DOWN TO HEADER WHERE OCCURS (POST WIDTH TO MATCH BEAM, NOT SHOWN FOR CLARITY). INSTALL FULL-DEPTH BLKG EACH SIDE OF JOIST OR BEAM OVER SUPPORT

FRAMING SCHEDULE

CALLOUT	JOIST/BREAM	HANGER (U.O.N. ON PLAN)	REFER TO DETAIL(S) (OR SEE NOTES BLW)
UFB2	1 1/2 x 11 1/8 LVL (FLUSH RIM BEAM)	HUS1.81/10	(E/S7)
DJ1	2x12 @16"o.c.	LRU212Z	(I/S7)
DB2	4x12 (FLUSH)	HUC410 (MANUF. SLOPED)	(F/S6) (E/S7)
DB3	5 1/2 x 13 1/2 PT GLB (DROPPED)	N/A	N/A
DJ4	2x8 @16"o.c.	N/A	(E/S6) (K/S6)
DH6	5 1/2 x 9 GLB (DROPPED HEADER)	N/A	(A/S2) USE DBL CRIPPLE STUDS EACH END
DH7	5 1/2 x 12 GLB (DROPPED HEADER)	N/A	(A/S2) USE DBL CRIPPLE STUDS EACH END
DB8	DBL 1 1/2 x 7 1/2 LVL (FLUSH w/ DJ4)	N/A	SIM (K/S6) (C/S2) INSTALL 'H8' EACH SIDE, EACH END TO NORTH FACE OF BEAM OR WALL BLW (PLACE o/ SHEATHING WHERE OCCURS)
EUFJ1	(E) 1 1/2 TJI @16"o.c.	N/A	N/A
EUFB2	(E) 3 1/2 x 1 1/8 PSL (FLUSH, V.I.F.)	N/A	STRAP EACH END DOWN TO (E) DBL TOP PLATE OR BEAM BLW w/ 'H8' EACH SIDE
EUFB3	(E) 6 1/2 x 18 GLB (DROPPED)	N/A	N/A



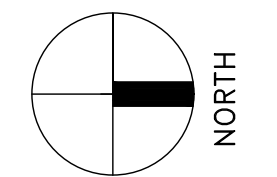
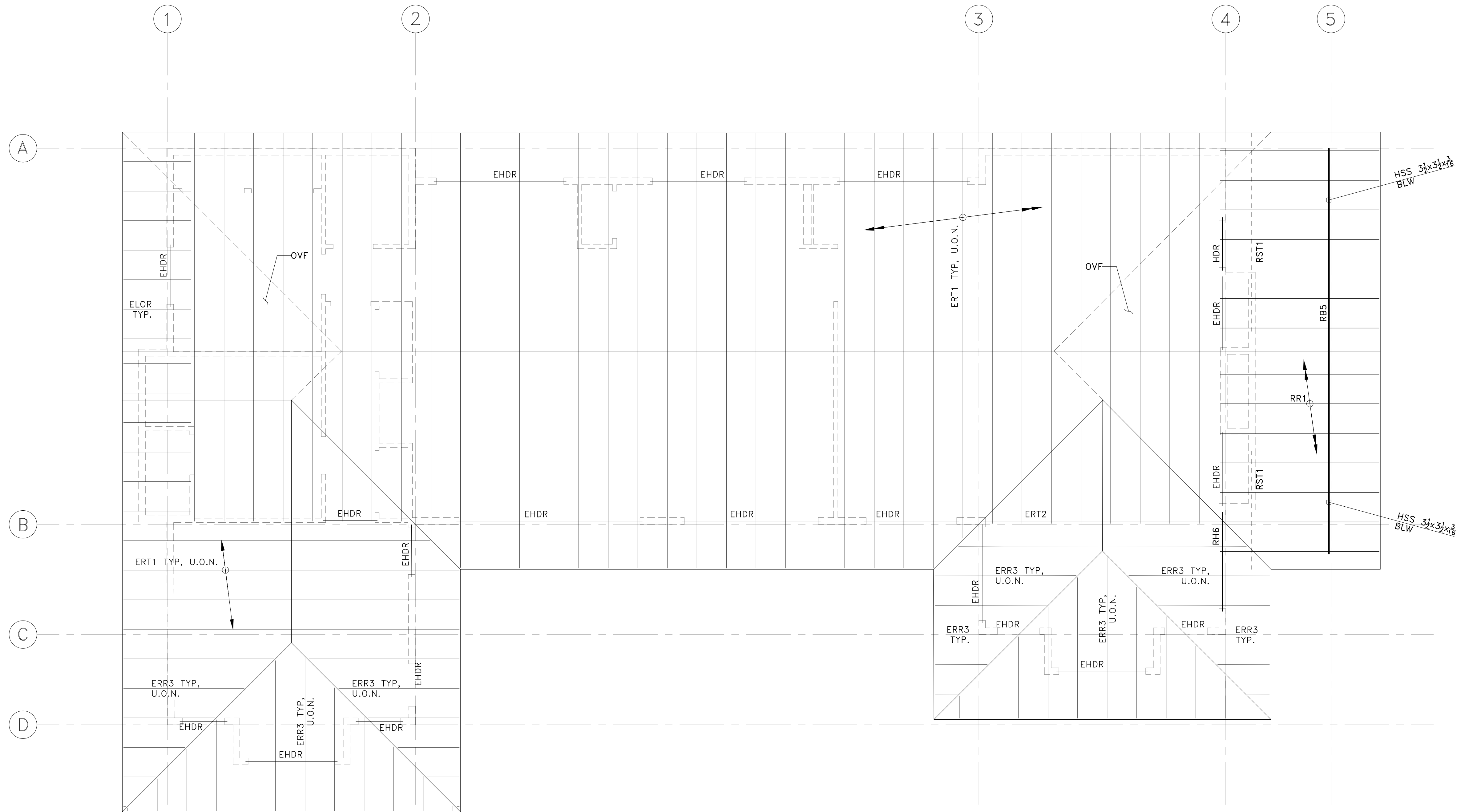
PERMIT SET	
REV	DATE
04-26-21	PERMIT SET
DESCRIPTION	
PROJECT: ADDITIONS & ALTERATIONS 3206 74th PI SE Mercer Island, WA 98040	
TEL: HAVN Architects 3206 74th PI SE Mercer Island, WA 98040	
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SHEET TITLE: UPPER FLOOR FRAMING PLAN	
SCALE: AS NOTED	SHEET NO. S4
JOB NO. 20025	

PLAN LEGEND

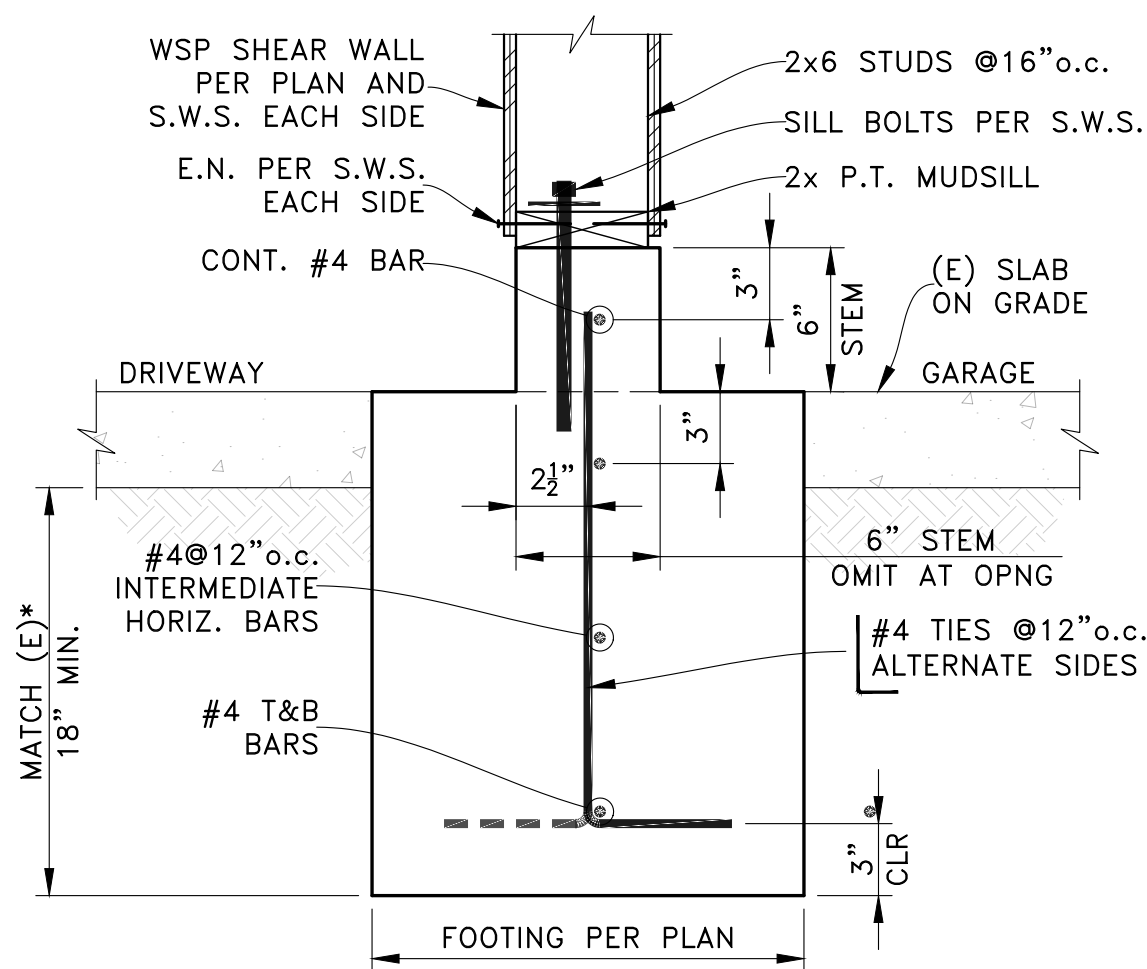
	NEW OR EXISTING WALL BELOW FLOOR
	POST BELOW FLOOR PER (F-C) S2
	METAL STRAP PER PLAN
RST1	CONT. "CS16" STRAP o/ SHEATHING o/ 2x4 FLAT BLKG; CONTINUE OVER 2x RIM BLKG ABOVE WALL FOR 4'-0" MIN.
HDR	DROPPED HEADER OVER WALL OPENING BLW PER (A) S2
OVF	OVER-FRAMING PER (H) S7
EHDR	(E) DROPPED HEADER OVER WALL OPENING BLW
ELOR	(E) LOOKOUT RAFTERS
	FLUSH-FRAMED JOIST OR BEAM CONNECTION; SEE FRAMING SCHEDULE FOR HANGERS, U.O.N. ON PLAN OR DETAILS (JOIST HANGERS NOT SHOWN ON PLAN FOR CLARITY)
	JOIST OR BEAM BEARING ON DROPPED BEAM OR HEADER (BEARING WALL SIM). POST DOWN TO HEADER WHERE OCCURS (POST WIDTH TO MATCH BEAM, NOT SHOWN FOR CLARITY). INSTALL FULL-DEPTH BLKG EACH SIDE OF JOIST OR BEAM OVER SUPPORT

FRAMING SCHEDULE

CALLOUT	JOIST/BAM	HANGER (U.O.N. ON PLAN)	REFER TO DETAIL(S) (OR SEE NOTES BLW)
RR1	2x6 @24"o.c.	N/A	(H) S6 (C) S7
RB5	(2) C10x15.3 (DROPPED)	N/A	(H) S6
RH6	(2) 1 1/2x7 LVL (DROPPED HEADER)	N/A	(A) USE TRPL CRIPPLE S2 STUDS BLW WEST END
ERT1	(E) MPCWT @24"o.c.	N/A	N/A
ERT2	(E) GIRDER TRUSS (V.I.F.)	N/A	N/A
ERR3	(E) RAFTERS @24"o.c.	N/A	N/A

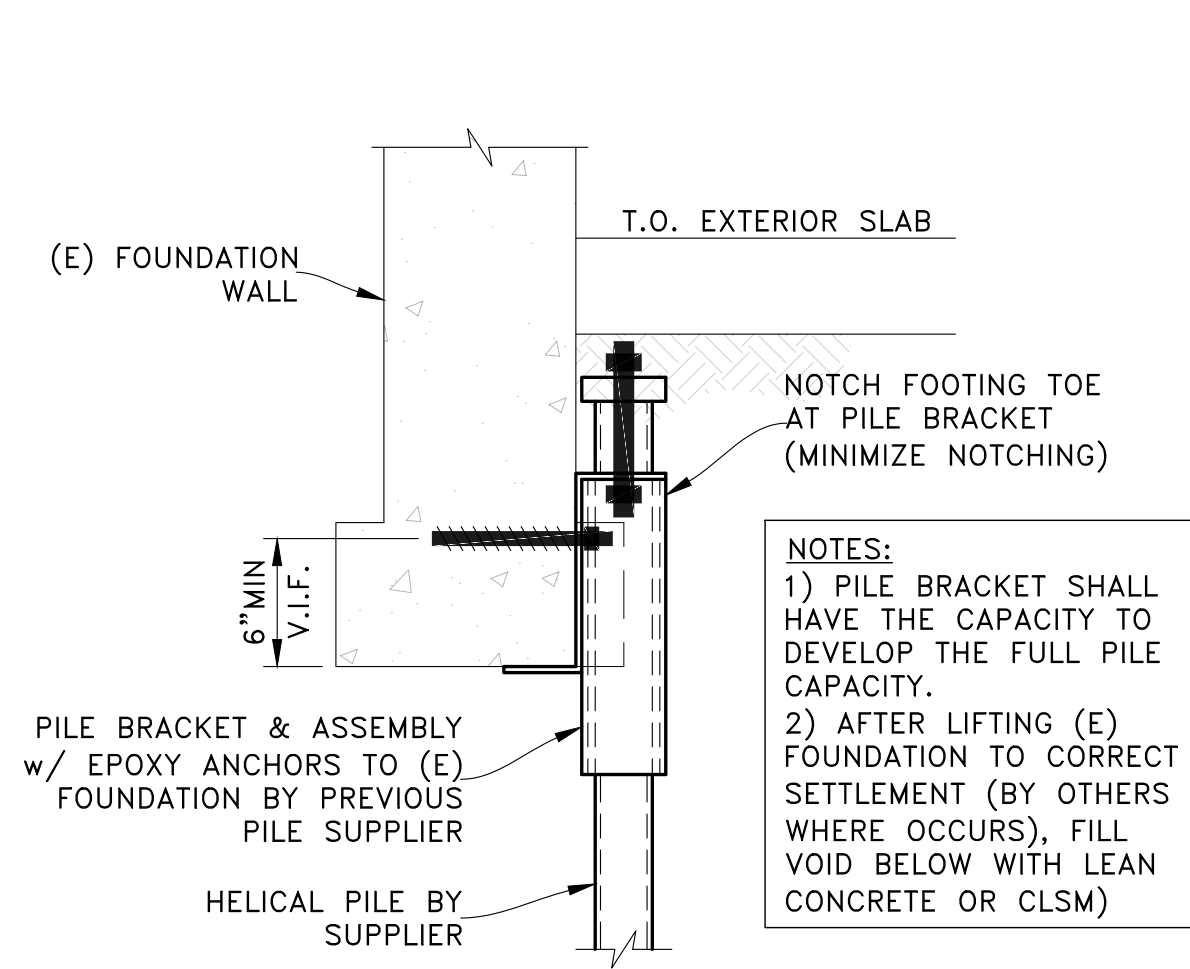
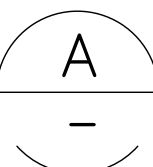


PERMIT SET	
REV	DATE
04-26-21	PERMIT SET
DESCRIPTION	
PROJECT: ADDITIONS & ALTERATIONS 3206 74th Pl SE Mercer Island, WA 98040	
TEL: HAVN Architects 3206 74th Pl SE Mercer Island, WA 98040	
ENGINEER OF RECORD	
O.G. ENGINEERING, PLLC 8645 22nd Ave SW, SEATTLE, WA 98106 (206) 290-4608 owen@ogengineer.com	
SHEET TITLE: ROOF FRAMING PLAN	
SCALE: AS NOTED	SHEET NO. S5
JOB NO. 20025	



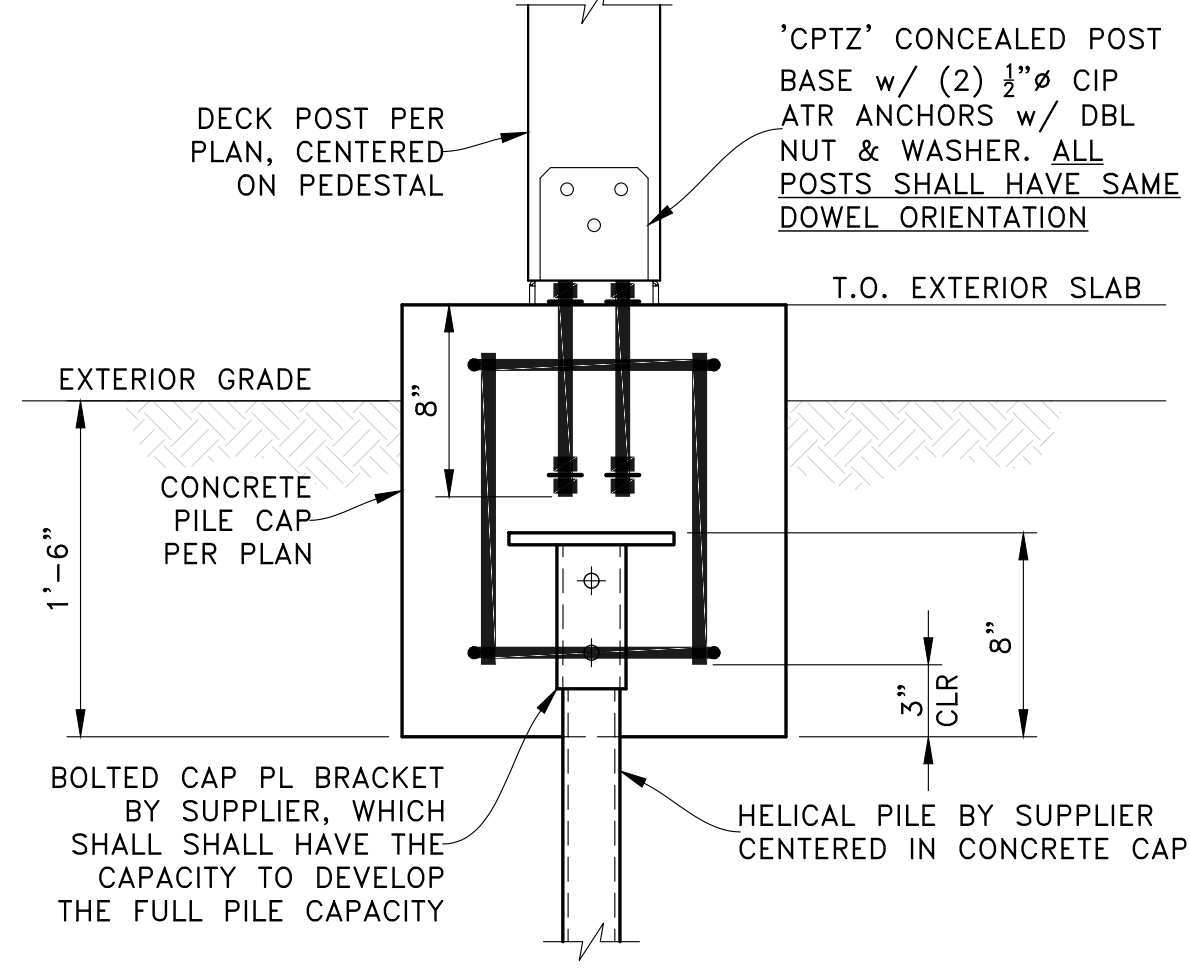
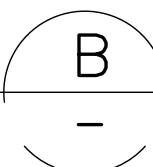
EXTERIOR STRIP FOOTING

SCALE: NTS



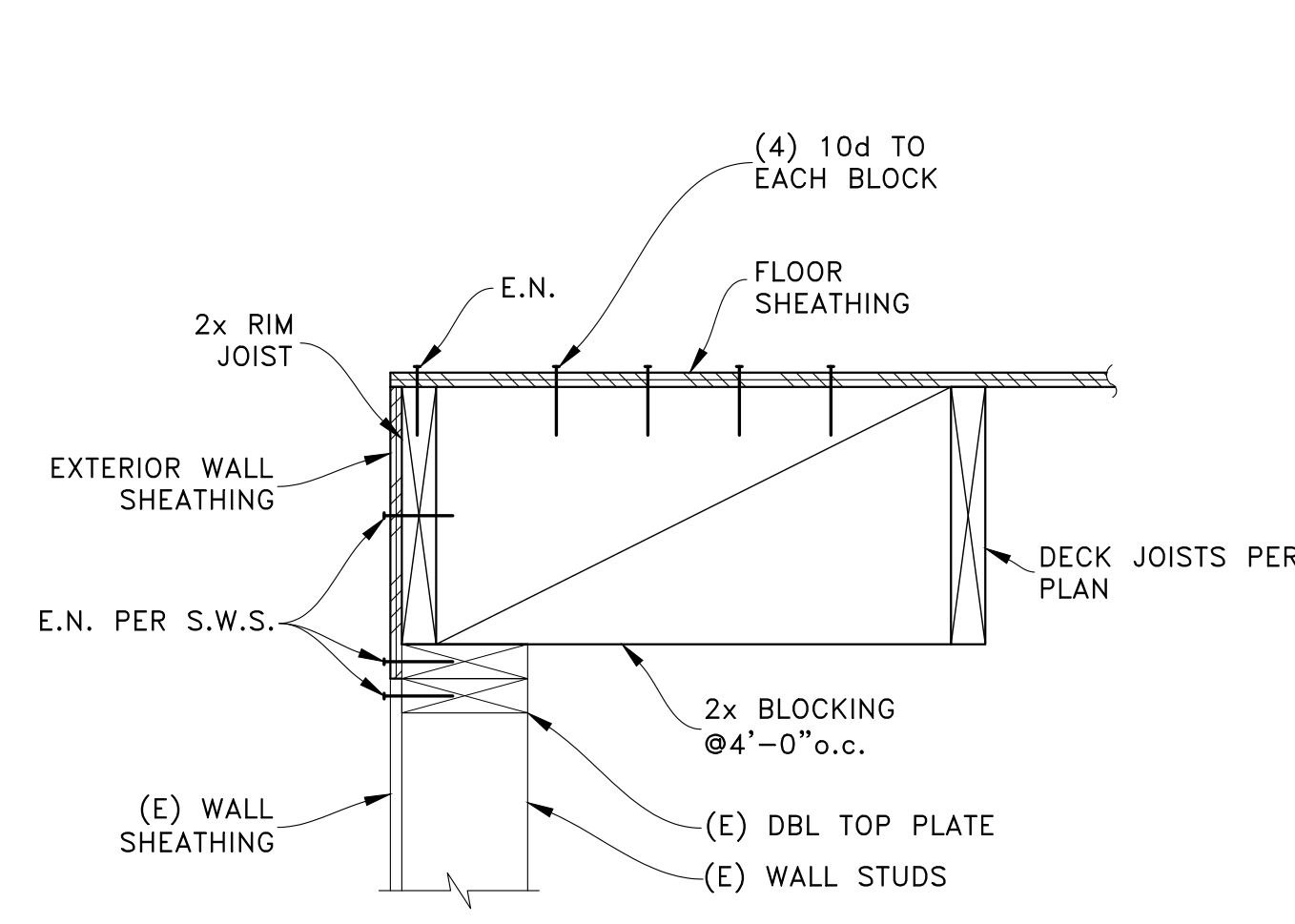
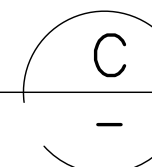
RETROFIT HELICAL PILE

SCALE: NTS



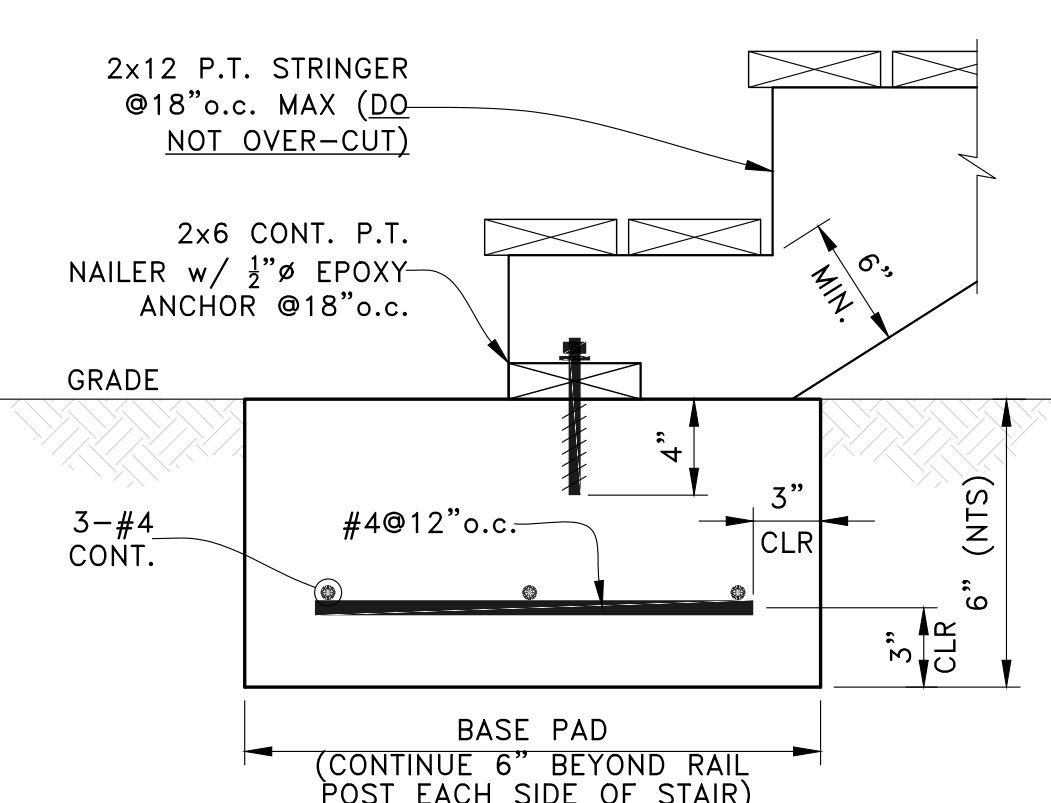
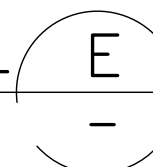
DECK POST PILE CAP

SCALE: NTS



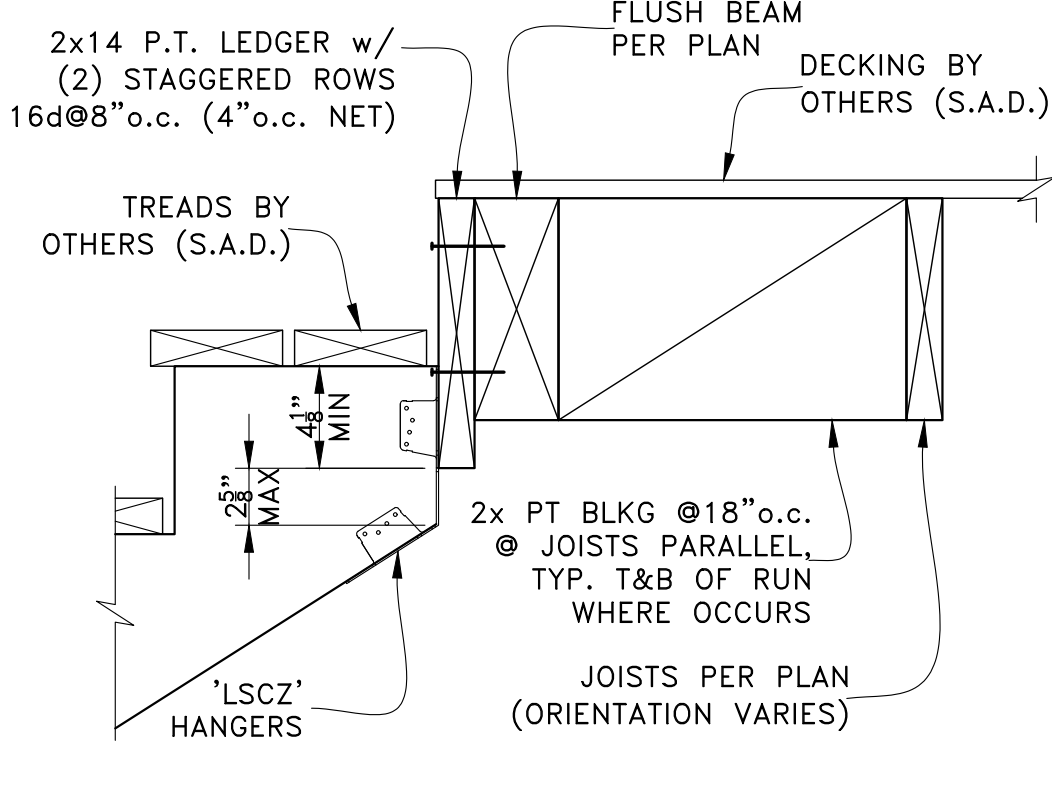
NORTH DECK AT EXISTING WALL

SCALE: NTS

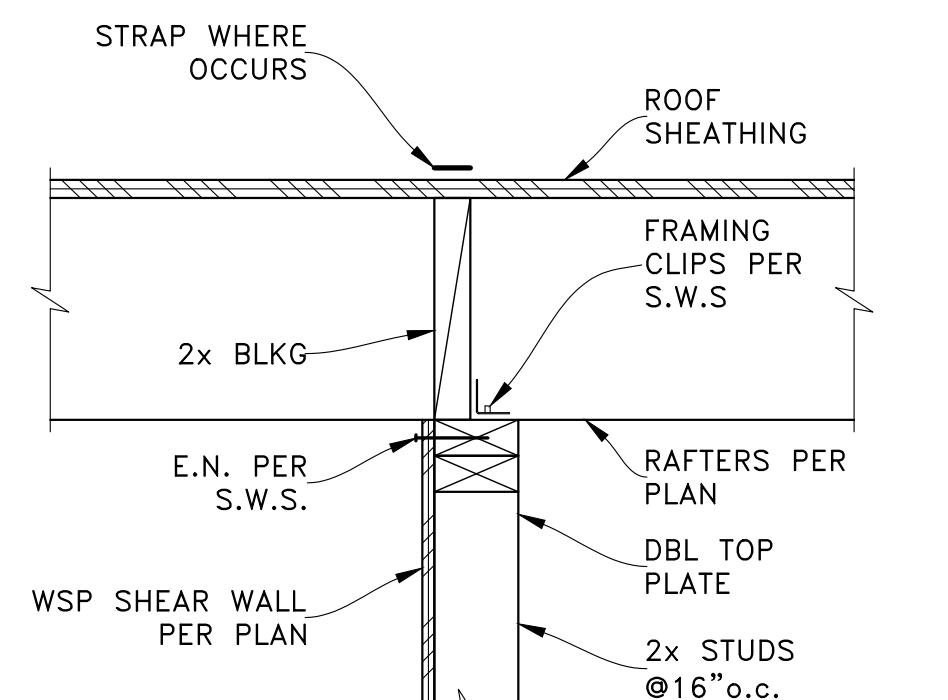
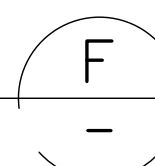


EXTERIOR STAIR

SCALE: NTS

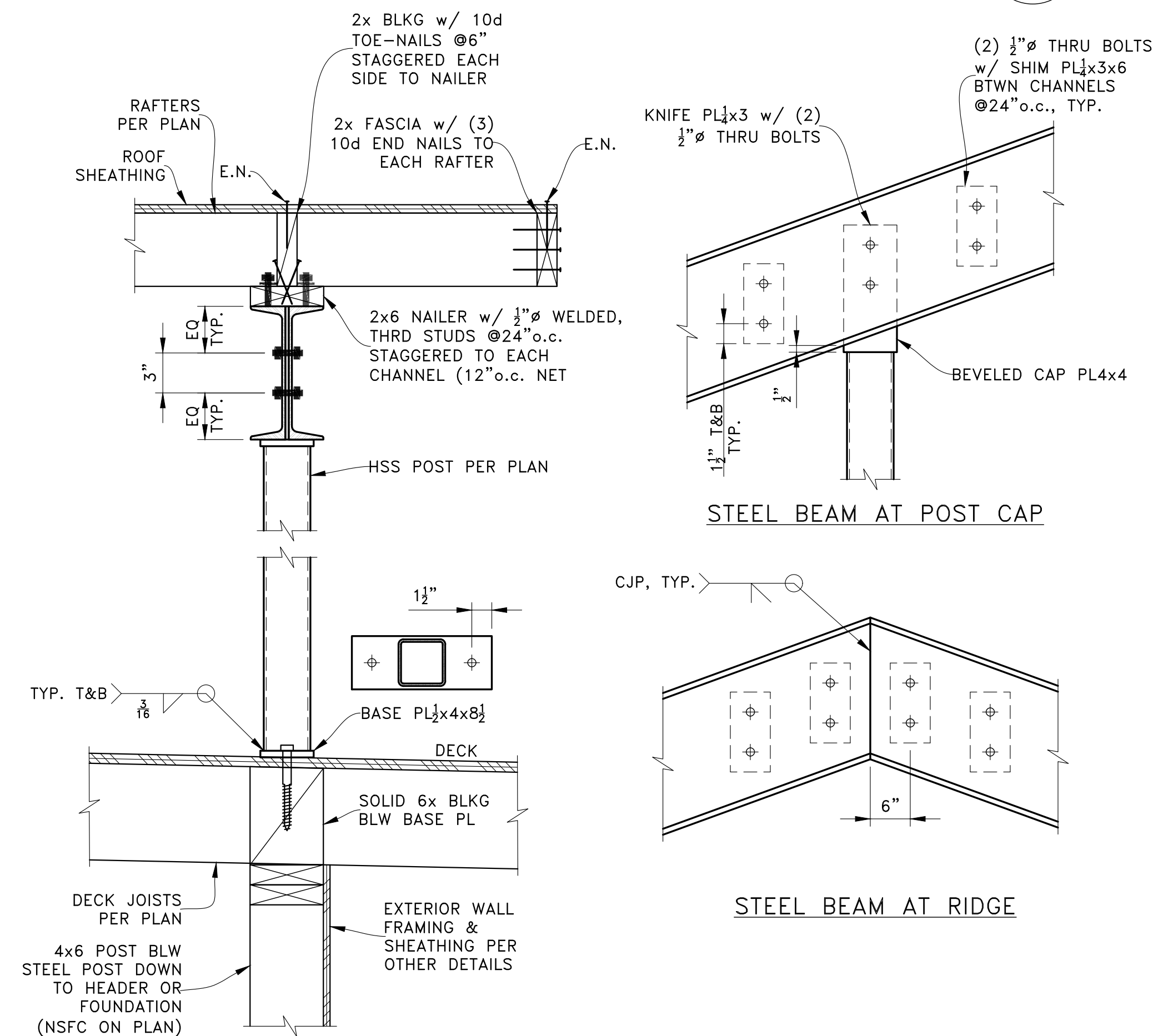
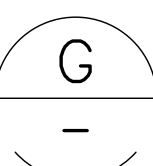


TOP OF RUN



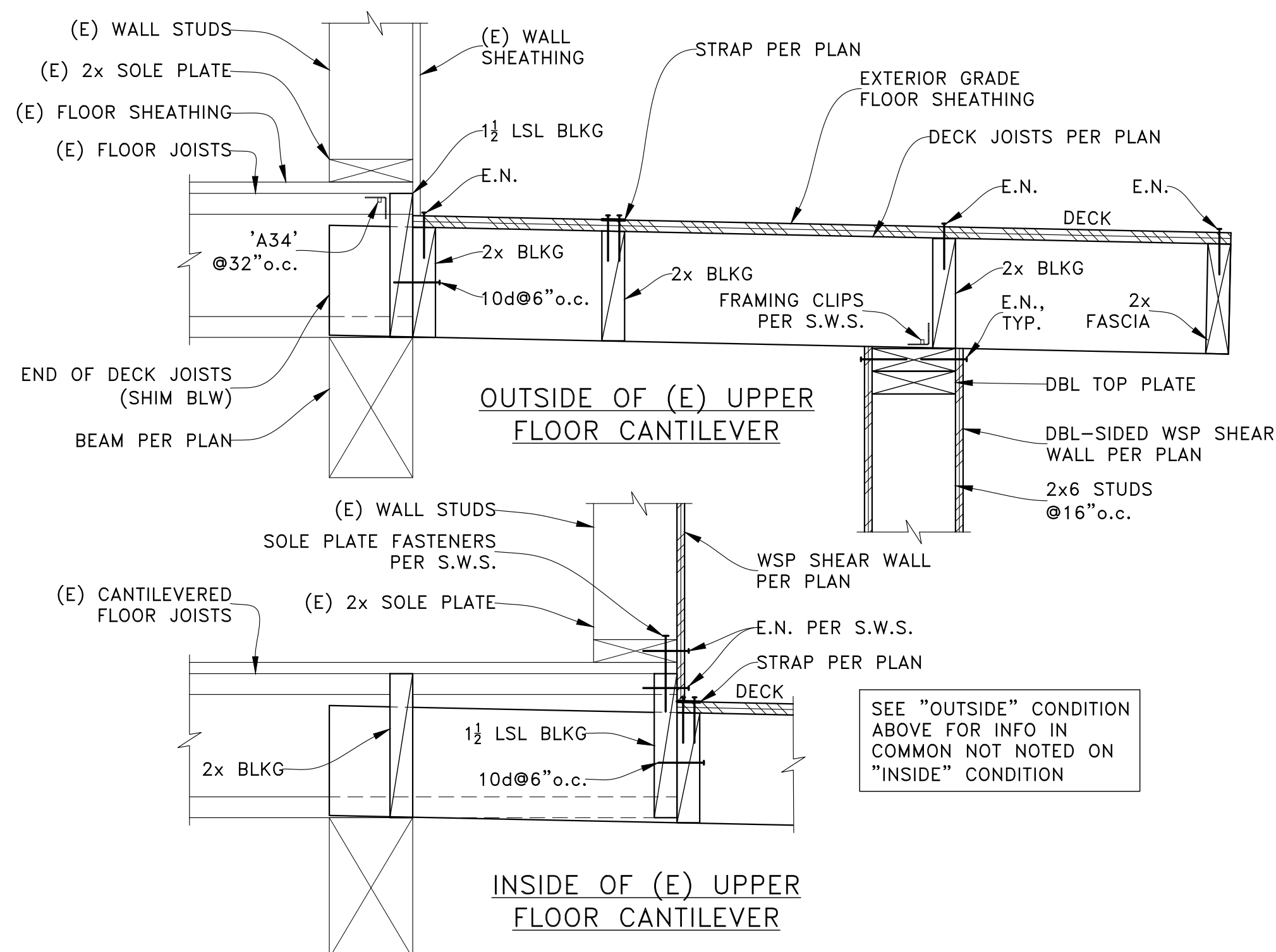
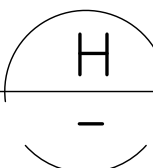
SHEAR WALL AT ROOF

SCALE: NTS



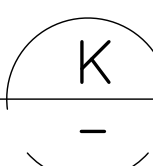
STEEL DECK BEAM & POST

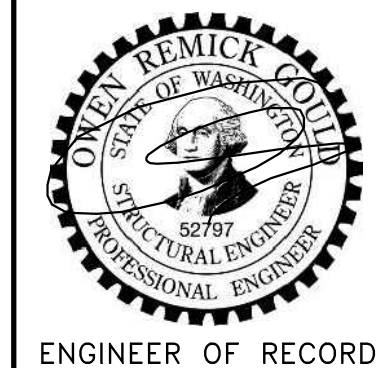
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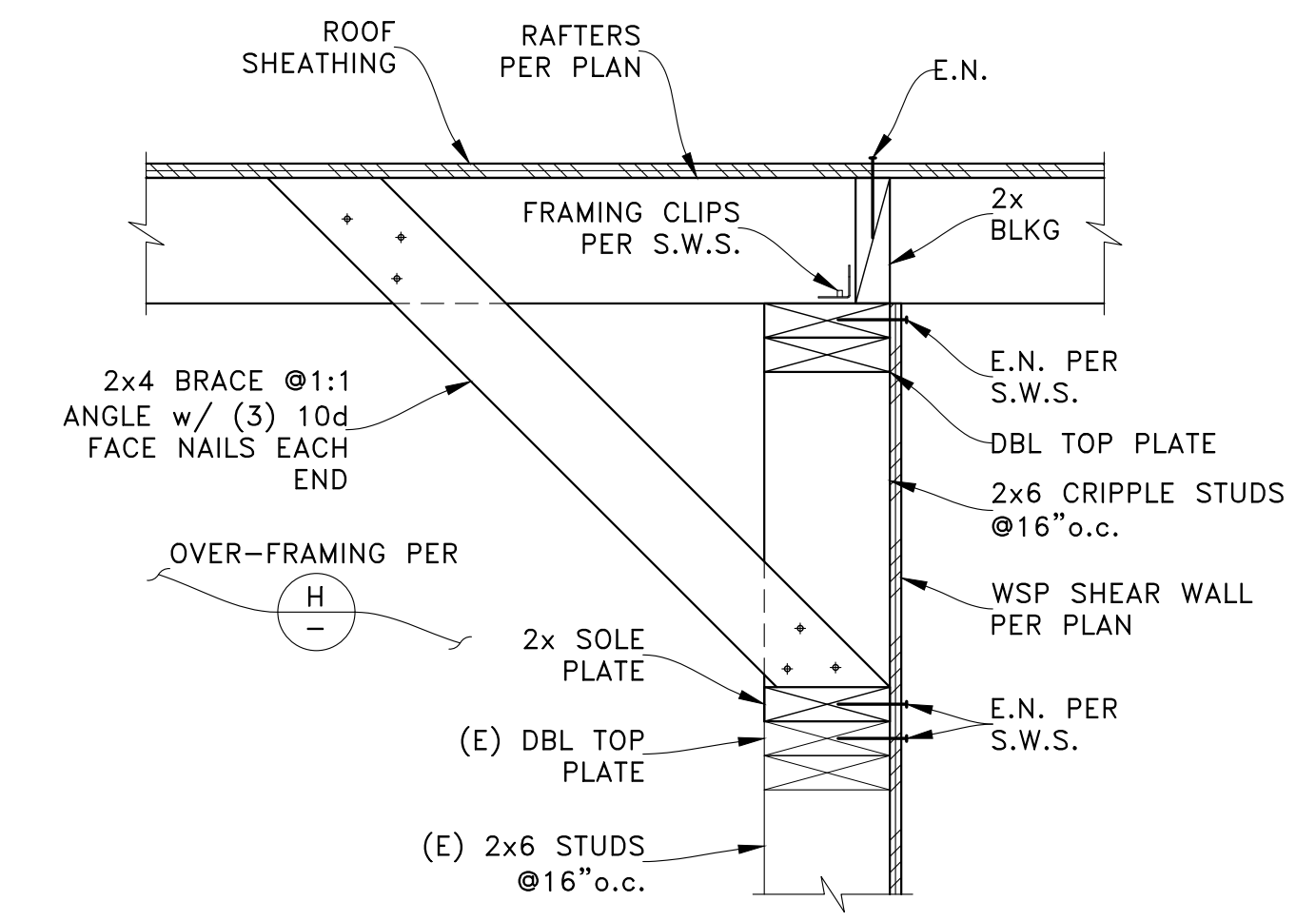


NORTH DECK

SCALE: NTS

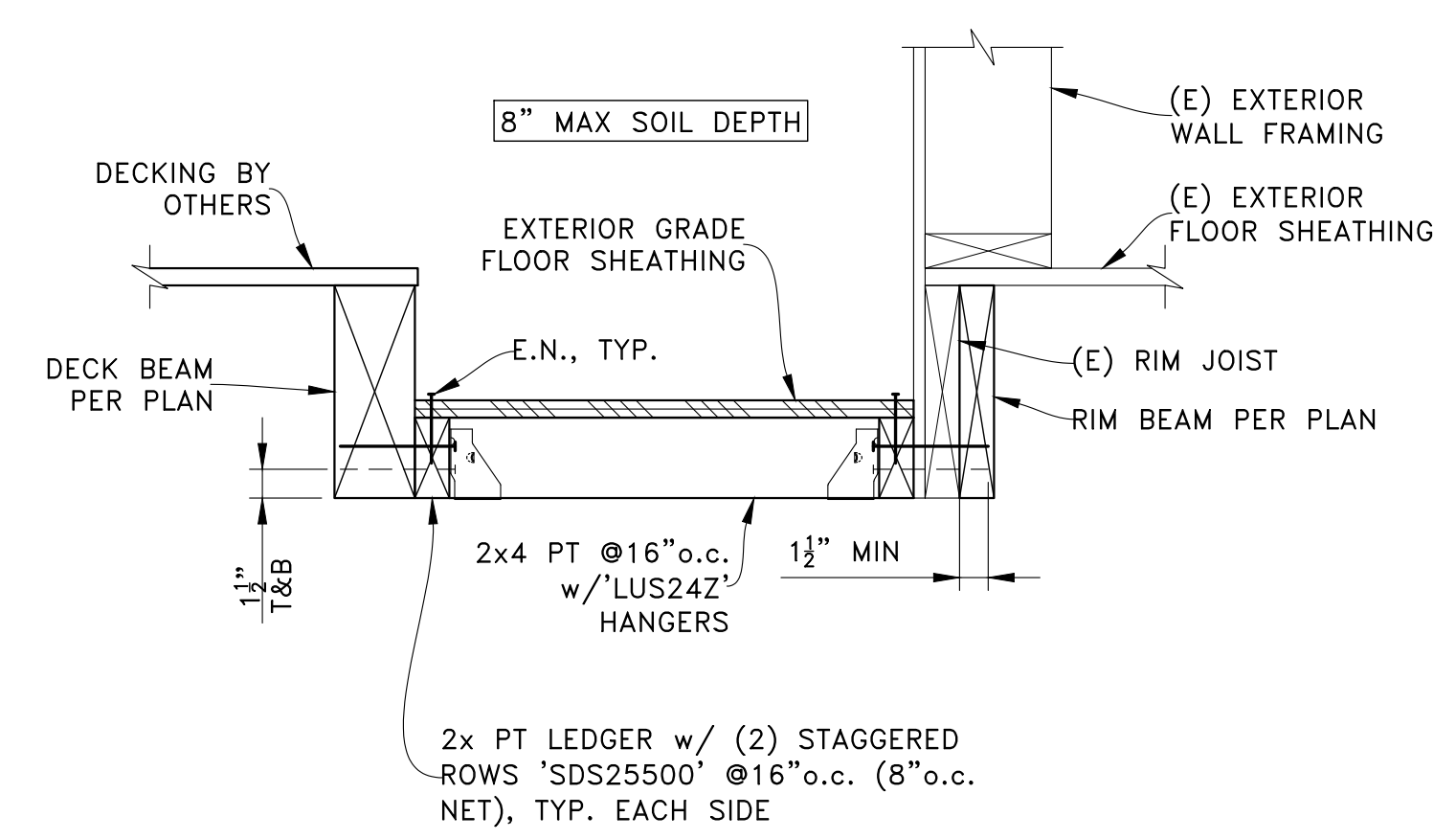
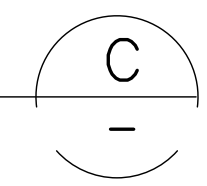


PERMIT SET	
04-26-21	PERMIT SET
04-26-21	DESCRIPTION
ADDITIONS & ALTERATIONS 3206 74th Pl SE Mercer Island, WA 98040 HAVN Architects 3206 74th Pl SE Mercer Island, WA 98040	
 OWEN REMICK STATE OF WASHINGTON PROFESSIONAL ENGINEER	
O.G. ENGINEERING, PLLC 8645 22nd Ave SW, SEATTLE, WA 98106 (206) 290-4608 owen@ogengineer.com	
SECTIONS & DETAILS	
SCALE: AS NOTED	SHEET NO. S6
JOB NO. 20025	



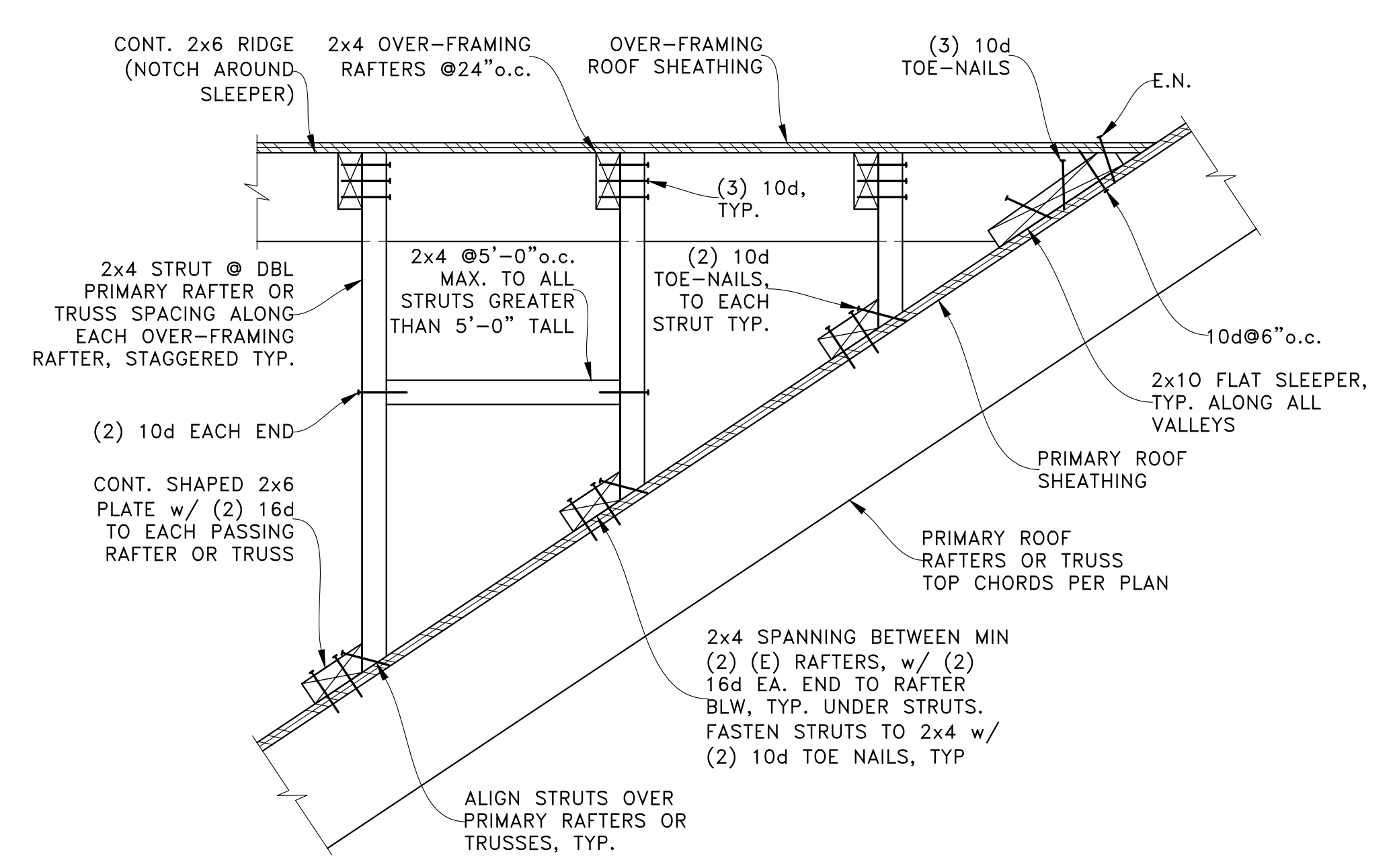
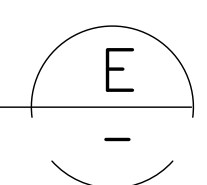
DECK ROOF AT EXTERIOR WALL

SCALE: NTS



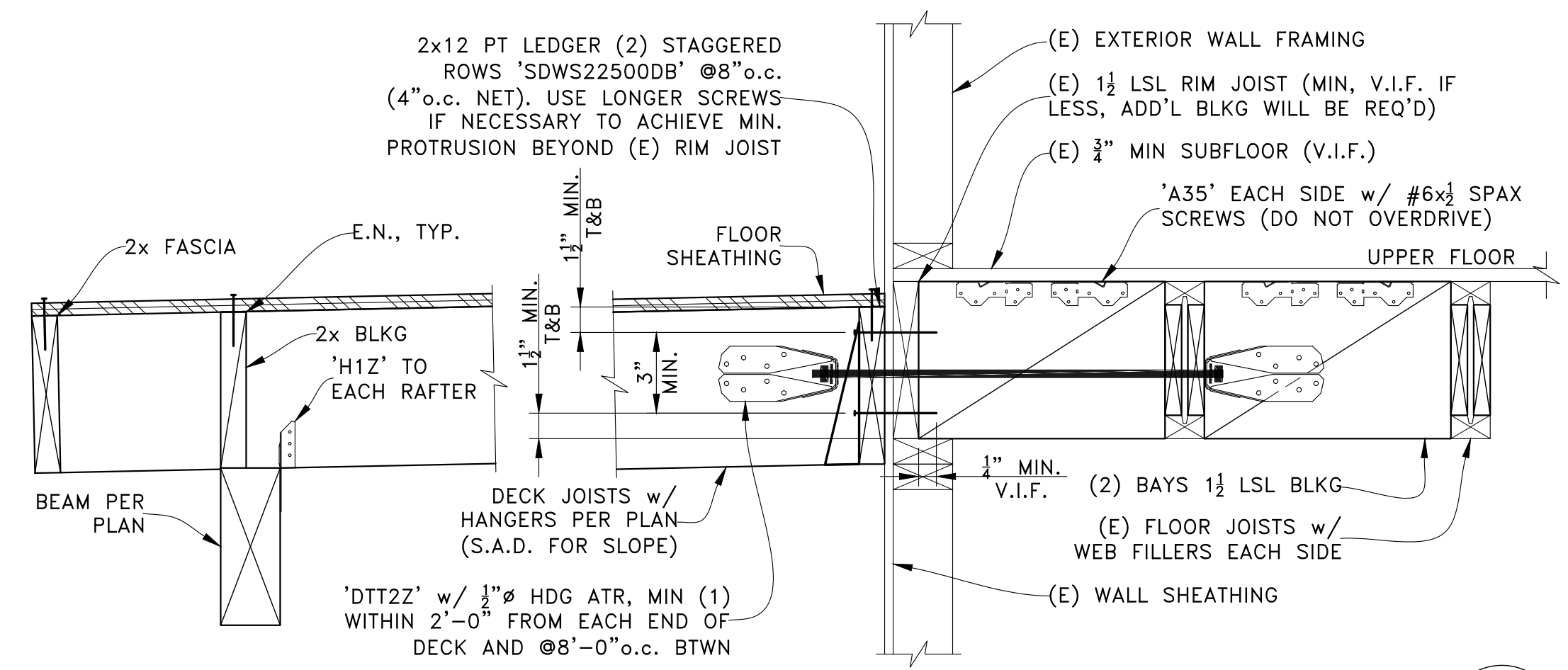
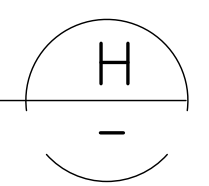
SUSPENDED PLANTER

SCALE: NTS



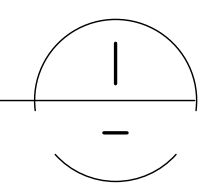
TYPICAL OVER-FRAMING

SCALE: NTS



EAST DECK

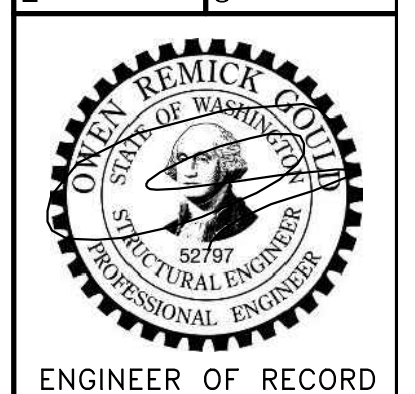
SCALE: NTS



PERMIT SET	
REV	DATE
04-26-21	PERMIT SET
	DESCRIPTION

PROJECT: ADDITIONS & ALTERATIONS
3206 74th PI SE
Mercer Island, WA 98040

CLIENT: HAVN Architects
3206 74th PI SE
Mercer Island, WA 98040



ENGINEER OF RECORD

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SHEET TITLE: SECTIONS & DETAILS

SCALE: AS NOTED	SHEET NO. S7
JOB NO. 20025	